

Università degli Studi di Pisa  
15 Febbraio 2010

## **PROBLEMI DI FRATTURA E DANNEGGIAMENTO IN MATERIALI E SISTEMI COMPOSITI**

**Parte 1:  
MECCANICA DELLA FRATTURA LINEARE E NON LINEARE NELLO  
STUDIO DEI MATERIALI COMPOSITI**

**Roberta Massabò**

Università degli Studi di Genova

# INDICE DELLA PRESENTAZIONE

## - Cenni di Meccanica delle Frattura Elastica Lineare

Concentrazione degli sforzi

Intensificazione degli sforzi - fattori di Intensificazione degli sforzi

Criterio di propagazione energetico

Forza generalizzata di propagazione della fessura

Frattura in condizioni di modo misto

Meccanica della frattura non lineare – il modello di Dugdale

## - Frattura in modo misto nei materiali compositi laminati

## - I modelli coesivo e bridged-crack nello studio della frattura nei compositi

Introduzione

Metodo delle weight functions

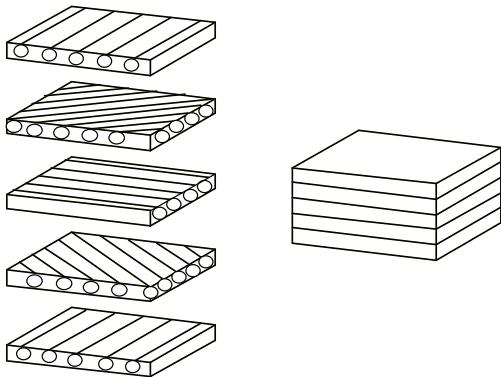
Lunghezze di scala e gruppi adimensionali

Transizioni nella risposta meccanica dei materiali compositi

Confronto tra i due metodi

## RICHIAMI DI MECCANICA DELLA FRATTURA ELASTICA LINEARE

# FIBER-REINFORCED COMPOSITE LAMINATES



## Lamina:

unidirectionally reinforced composite

## Laminate:

stacking sequences:  $[0]_n$ ,  $[\pm\alpha]_n$ ,  $[0/90]_n$ ,  $[0/\pm45/90]_n$ , ...

## Manufacturing procedures:

- lay-up
- resin impregnation (in PMC): RTM, RFI

## CHARACTERISTICS:

- High degree of tailoring of elastic properties
- High strength to weight ratios
- High stiffness to weight ratios  
( $\Rightarrow$  aeronautics/aerospace/defence)

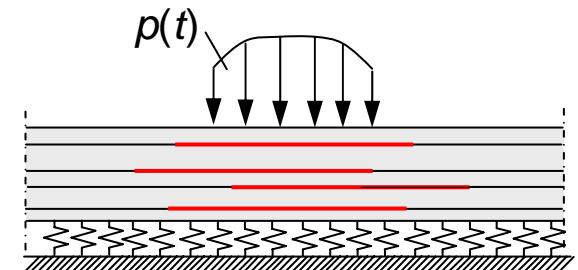
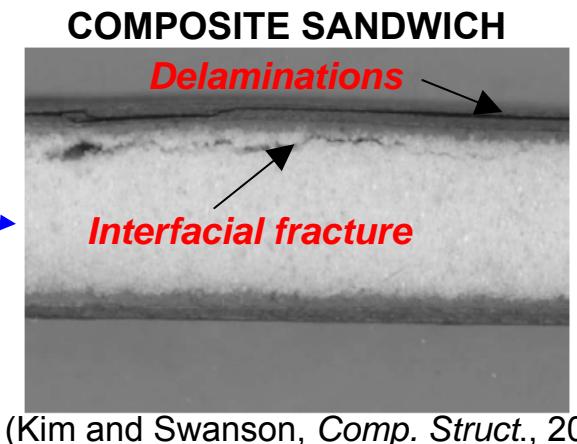
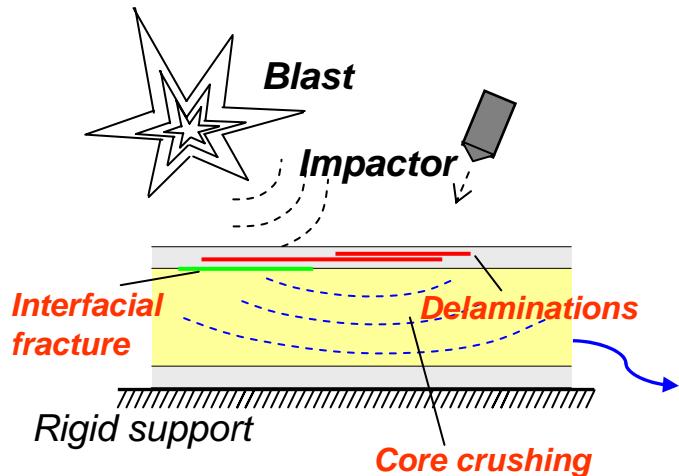
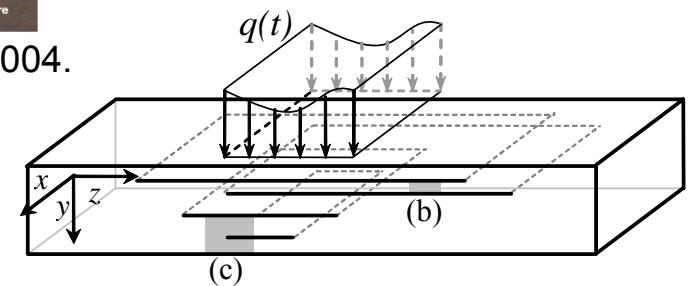
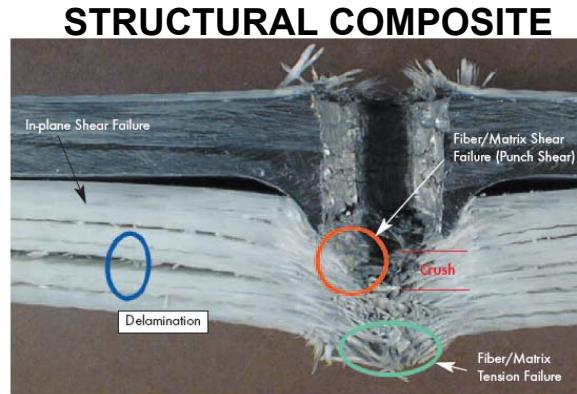
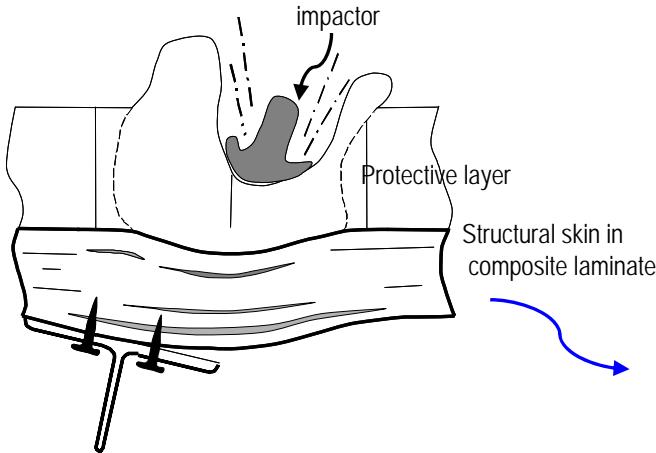
## DRAWBACKS:

- Lack of strength in through-thickness direction
- Low damage / impact tolerance and resistance
- High sensitivity to interlaminar flaws
- Catastrophic failures  
(inter-ply layers: low-toughness fracture paths )

## REMEDIES:

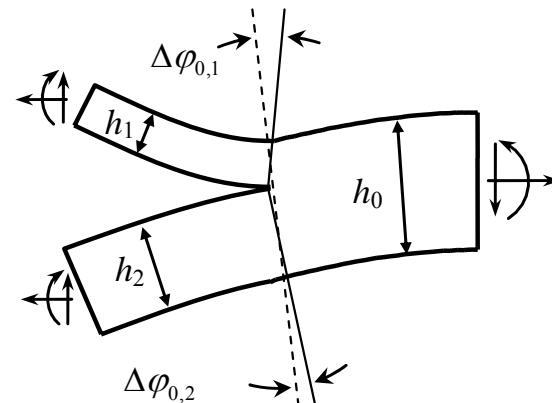
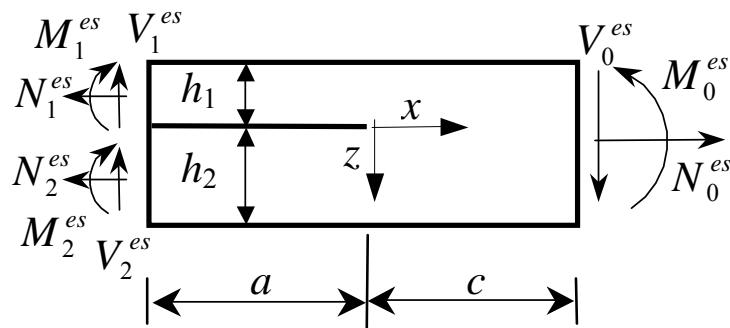
- Tougher matrices
- Inter-ply films, addition of particles,...
- Through-thickness reinforcement (trans-laminar reinforcement)

# SINGLE AND MULTIPLE DELAMINATION



# THE EFFECTS OF SHEAR AND NEAR TIP DEFORMATIONS ON ENERGY RELEASE RATE AND MODE MIXITY OF EDGE-CRACKED ORTHOTROPIC LAYERS

(M.G. Andrews & R. Massabò, 2007, *Engineering Fracture Mechanics*)



**Edge cracked layer subject to generalized end forces**

**Reference solutions from the literature:**

**Suo, JAM, (1990):**

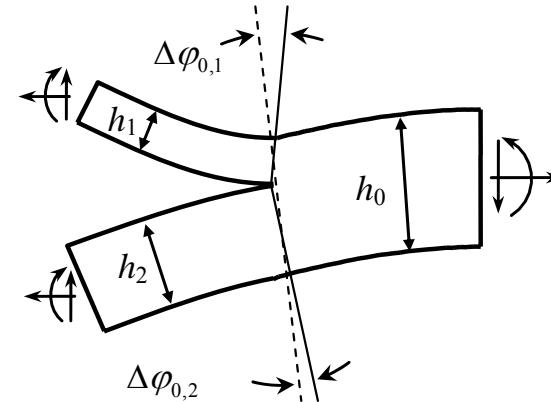
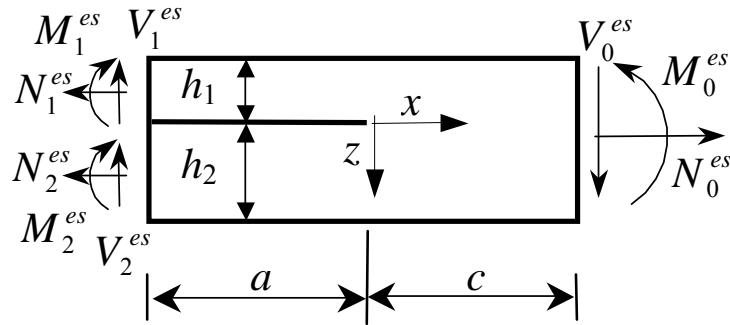
- axial loading only (bending moments and normal forces)
- analytical expression for energy release rate,  $\equiv$  elementary beam theory
- analytical mode decomposition based on dimensional considerations, linearity, relationship between ERR and SIF (Irwin, Sih)
- (derivation is analytical except for a single parameter extracted from numerical solution of one loading case)

**Li, Wang and Thouless,  
JMPS, (2004):**

- accounts for effects of shear in bimaterial isotropic beams
- numerical FE solution for the energy release rate
- expressions for SIFs depending on 5 numerically determined constants

# THE EFFECTS OF SHEAR AND NEAR TIP DEFORMATIONS ON ENERGY RELEASE RATE AND MODE MIXITY OF EDGE-CRACKED ORTHOTROPIC LAYERS

(M.G. Andrews & R. Massabò, 2007, *Engineering Fracture Mechanics*)

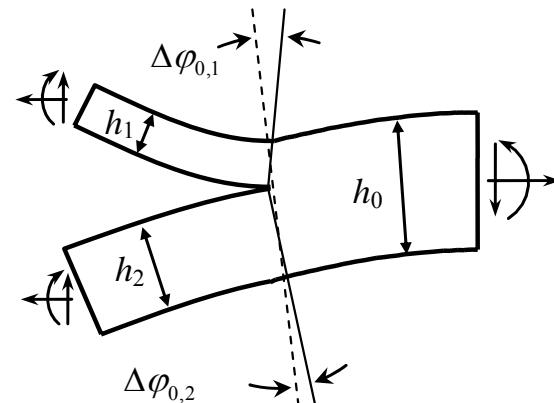
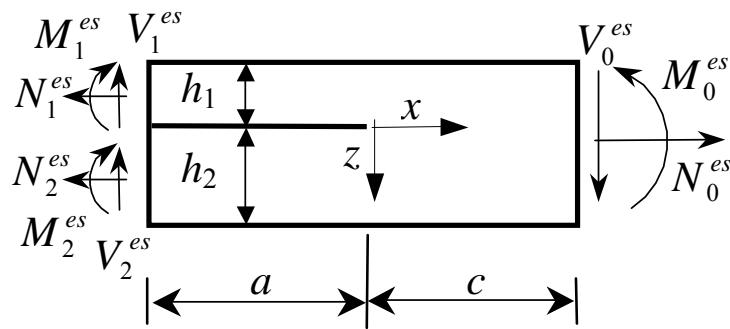


**Edge cracked homogeneous and orthotropic linear elastic layer  
subject to generalized end forces**

- Plane problem
- x and z = principal material axes
- Non dimensional orthotropy ratios (plane stress):  $\lambda = \frac{E_z}{E_x}$ ,  $\rho = \frac{\sqrt{E_x E_z}}{2G_{xz}} - \sqrt{\nu_{zx} \nu_{xz}}$   
 $0 < \rho < 5$  and  $0 < \lambda < 1$  (typical values for composites)
- $a, c \geq c_{\min} = h_i \lambda^{-1/4}$  ( $i = 0, 1, 2$ ):  
**so that crack tip fields independent of distance from load points**

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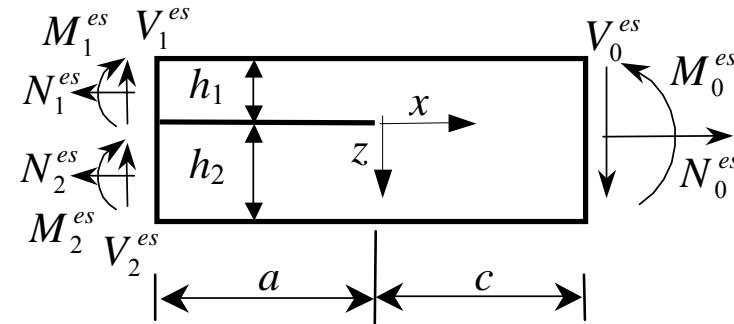
- Plane problem

-  $x$  and  $z$  Aim of our work:

- Non-dimensional parameter  $\rho = \frac{c}{a}$ ,  $0 < \rho < 5$   
- Derive semi-analytical expressions for the energy release rate and the stress intensity factors in homogeneous orthotropic layers that depend on the crack tip stress resultants;

-  $a, c \geq 0$  so that  $\rho \leq 5$   
- The expressions have physical significance and allow separation of the different contributions

# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS



**Self equilibrated crack tip loading systems:**

$$\begin{array}{c}
 \text{Diagram (a)} \\
 \text{Left: } M_1, V_1, h_1, h_2, N_1, N_2, M_2, V_2, \Delta x \rightarrow 0 \\
 \text{Right: } M_0, V_0, N_0
 \end{array} = 
 \begin{array}{c}
 \text{Diagram (b)} \\
 \text{Left: } M_0, h_1, h_2, N_0 \\
 \text{Right: } M_0, N_0
 \end{array} + 
 \begin{array}{c}
 \text{Diagram (c)} \\
 \text{Left: } M, N, h_1, h_2, M^* \\
 \text{Right: } M^*, N
 \end{array} + 
 \begin{array}{c}
 \text{Diagram (d)} \\
 \text{Left: } V_S, h_1, h_2 \\
 \text{Right: } V_S
 \end{array} + 
 \begin{array}{c}
 \text{Diagram (e)} \\
 \text{Left: } V_D, h_1, h_2 \\
 \text{Right: } V_D
 \end{array}$$

$$M^* = M + N \frac{h_1 + h_2}{2}$$

$$R = [M/E_x h_1^2; N/E_x h_1; V_S/E_x h_1; V_D/E_x h_1].$$

## ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

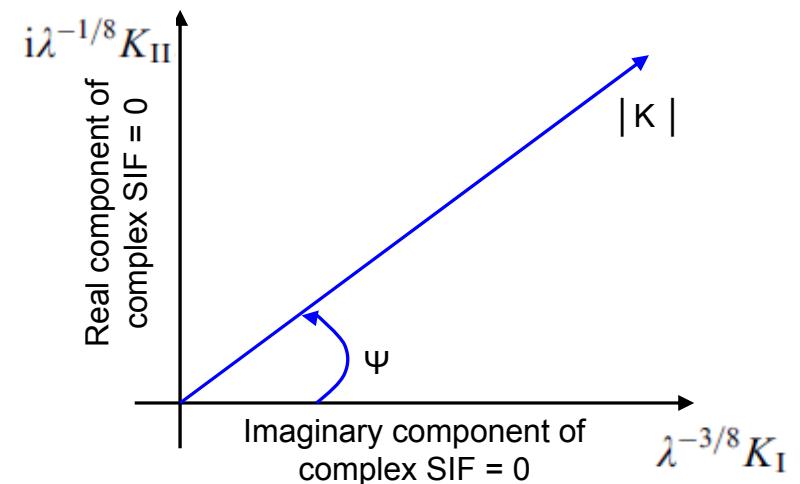
Relationship between energy release rate and stress intensity factors in orthotropic body (Sih, 1965):

$$\mathcal{G} = \frac{1}{E_x} \sqrt{\frac{1+\rho}{2}} [\lambda^{-3/4} K_I^2 + \lambda^{-1/4} K_{II}^2],$$

Complex stress intensity factor:

$$K = \lambda^{-3/8} K_I + i\lambda^{-1/8} K_{II},$$

$$|K| = \sqrt{\lambda^{-3/4} K_I^2 + \lambda^{-1/4} K_{II}^2},$$



$$\Rightarrow \quad \sqrt{E_x \mathcal{G}} = \left( \frac{1+\rho}{2} \right)^{1/4} |K|. \quad (*)$$

Dimensional considerations, linearity and Eq. (\*) :

$$|K_R| = f_R(\eta, \lambda, \rho) R E_x \sqrt{h_1} \left( \frac{1+\rho}{2} \right)^{-1/4}, \quad f_R^2 = \mathcal{G}_R / (R^2 E_x h_1).$$

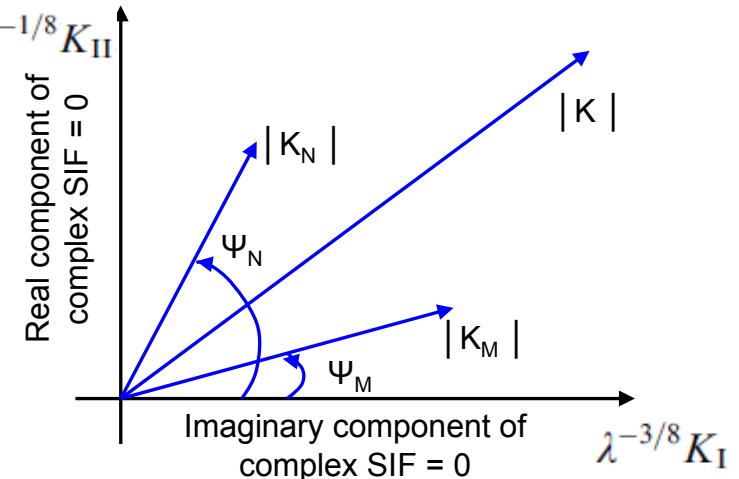
# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

**Components of complex stress intensity factor:**

$$K_{IR} = \lambda^{3/8} |K_R| \cos(\Psi_R)$$

$$K_{IIR} = \lambda^{1/8} |K_R| \sin(\Psi_R)$$

$$\Psi = \tan^{-1} \left( \lambda^{1/4} \frac{K_{II}}{K_I} \right).$$



**Assuming:**

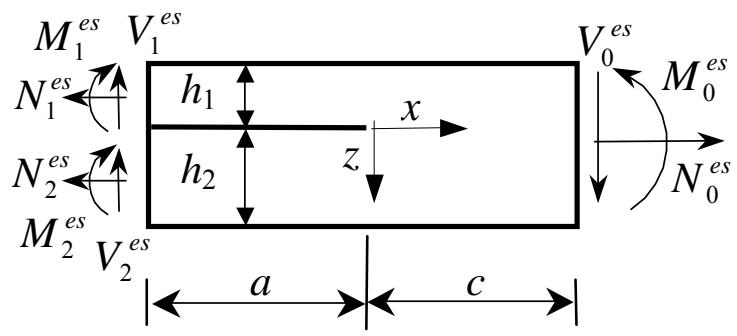
$$\Psi_N = \omega, \quad \Psi_M = \gamma_M + \omega - \frac{\pi}{2}, \quad \Psi_{V_D} = \gamma_{V_D} + \omega - \frac{\pi}{2}, \quad \Psi_{V_S} = \gamma_{V_S} + \omega - \frac{\pi}{2}.$$

**Stress Intensity Factors:**

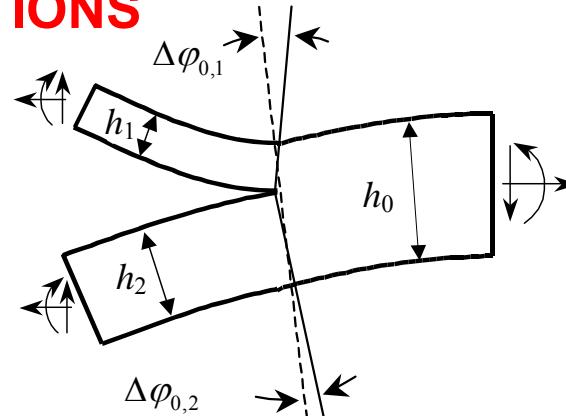
$$K_I = \frac{\lambda^{3/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ \frac{f_M M \sin(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \cos(\omega)}{\sqrt{h_1}} + \frac{f_{V_D} V_D \sin(\gamma_{V_D} + \omega)}{\sqrt{h_1}} + \frac{f_{V_S} V_S \sin(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right],$$

$$K_{II} = \frac{\lambda^{1/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ -\frac{f_M M \cos(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \sin(\omega)}{\sqrt{h_1}} - \frac{f_{V_D} V_D \cos(\gamma_{V_D} + \omega)}{\sqrt{h_1}} - \frac{f_{V_S} V_S \cos(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right].$$

## ROOT ROTATIONS



$$\begin{array}{c}
 \text{Diagram showing a beam element with two segments of heights } h_1 \text{ and } h_2. \text{ The beam is fixed at the bottom left. At the top left, there are end moments } M_1^e \text{ and } N_1^e, \text{ and end forces } V_1^e \text{ and } N_2^e. \text{ At the top right, there are end moments } M_0^e \text{ and } N_0^e, \text{ and end forces } V_0^e \text{ and } N_0^e. \text{ A coordinate system } (\Delta x \rightarrow 0) \text{ is shown at the center of the beam.} \\
 = \quad \begin{array}{c} M_0 \\ N_0 \end{array} + \begin{array}{c} M \\ N \end{array} + V_S \uparrow \quad V_D \uparrow \\
 \text{(a)} \qquad \qquad \qquad M^* = M + N \frac{h_1 + h_2}{2}
 \end{array}$$



### Root rotations:

$$\begin{aligned}
 \Delta\varphi_{0,1} &= \frac{1}{E_x h_1} \left( \frac{a_1^M}{h_1} M + a_1^N N + a_1^{V_s} V_s + a_1^{V_D} V_D \right) \\
 \Delta\varphi_{0,2} &= \frac{1}{E_x h_1} \left( \frac{a_2^M}{h_1} M + a_2^N N + a_2^{V_s} V_s + a_2^{V_D} V_D \right)
 \end{aligned}$$

and:  $\eta = h_1/h_2$     $\lambda = E_z/E_x$     $\rho = \frac{\sqrt{E_x E_z}}{2G_{xz}} - \sqrt{\nu_{zx} \nu_{xz}}$

**where:**

$$\begin{aligned}
 a_1^M, a_2^M, a_1^N, a_2^N &= b\lambda^{-1/4} \\
 a_1^{V_D} &= b\lambda^{-1/2} - \nu_{xz}/\kappa_s \\
 a_2^{V_D} &= b\lambda^{-1/2} + \eta\nu_{xz}/\kappa_s \\
 a_1^{V_s} &= b\lambda^{-1/2} - 1/(1+\eta)\nu_{xz}/\kappa_s \\
 a_2^{V_s} &= b\lambda^{-1/2} + \eta/(1+\eta)\nu_{xz}/\kappa_s
 \end{aligned}$$

# ROOT ROTATIONS

Table 1

Root rotation compliance coefficients of an orthotropic edge cracked layer with arbitrary generalized end forces acting at a distance  $\gg c_{\min} = h_i \lambda^{-1/4}$  ( $i = 0, 1, 2$ ) from the crack tip; results apply to any  $a, c \geq c_{\min} = h_i \lambda^{-1/4}$  with uncertainties lower than  $\pm 2\%$

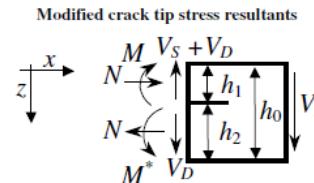
$$\Delta\varphi_{0,1} = \frac{1}{E_x h_1} \left( \frac{a_1^M}{h_1} M + a_1^N N + a_1^{V_s} V_S + a_1^{V_D} V_D \right) \quad \Delta\varphi_{0,2} = \frac{1}{E_x h_1} \left( \frac{a_2^M}{h_1} M + a_2^N N + a_2^{V_s} V_S + a_2^{V_D} V_D \right)$$

$$a_1^M, a_2^M, a_1^N, a_2^N = b \lambda^{-1/4},$$

$$a_1^{V_D} = b \lambda^{-1/2} - v_{xz}/\kappa_S, \quad a_2^{V_D} = b \lambda^{-1/2} + \eta v_{xz}/\kappa_S$$

$$a_1^{V_s} = b \lambda^{-1/2} - \frac{1}{1 + \eta \kappa_S} v_{xz}, \quad a_2^{V_s} = b \lambda^{-1/2} + \frac{\eta}{1 + \eta \kappa_S} v_{xz}$$

$$\text{with } \lambda = \frac{E_z}{E_x}, \rho = \frac{\sqrt{E_x E_z}}{2G_{xz}} - \sqrt{v_{zx} v_{xz}} \text{ and } \kappa_S = 5/6$$



$$M^* = M + N(h_1 + h_2)/2$$

Constant $b$								
for:	$a_1^M$	$a_2^M$	$a_1^N$	$a_2^N$	$a_1^{V_D}$	$a_2^{V_D}$	$a_1^{V_s}$	$a_2^{V_s}$
$\eta = h_1/h_2$	$\rho = 1$ and $0.025 \leq \lambda \leq 1$							
→ 0.0	7.439	0.000	1.732	0.000	2.606	0.000	2.606	0.000
0.2	7.529	-0.372	2.066	-0.331	2.188	-0.137	2.208	-0.146
0.4	7.672	-1.399	2.304	-1.077	1.945	-0.410	1.758	-0.350
0.6	7.817	-3.034	2.492	-2.163	1.769	-0.746	1.382	-0.506
0.8	7.952	-5.262	2.649	-3.567	1.631	-1.120	1.087	-0.604
1.0	8.077	-8.077	2.782	-5.280	1.518	-1.518	0.858	-0.656
$\eta$	$\rho = 3$ and $0.025 \leq \lambda \leq 1$							
→ 0.0	10.046	0.000	2.107	0.000	2.825	0.000	2.825	0.000
0.2	10.234	-0.480	2.604	-0.475	2.237	-0.126	2.684	-0.184
0.4	10.487	-1.860	2.935	-1.544	1.943	-0.384	2.200	-0.408
0.6	10.705	-4.094	3.183	-3.087	1.743	-0.709	1.716	-0.526
0.8	10.884	-7.156	3.379	-5.069	1.591	-1.074	1.306	-0.544
1.0	11.029	-11.029	3.541	-7.472	1.469	-1.469	0.977	-0.488
$\eta$	$\rho = 5$ and $0.025 \leq \lambda \leq 1$							
→ 0.0	12.029	0.000	2.408	0.000	2.931	0.000	2.931	0.000
0.2	12.313	-0.566	3.043	-0.586	2.226	-0.115	3.105	-0.221
0.4	12.663	-2.225	3.449	-1.902	1.904	-0.357	2.613	-0.463
0.6	12.946	-4.927	3.745	-3.796	1.692	-0.668	2.034	-0.542
0.8	13.163	-8.637	3.975	-6.221	1.533	-1.022	1.518	-0.475
1.0	13.331	-13.331	4.161	-9.153	1.406	-1.406	1.093	-0.308

Uncertainties estimated on root rotations from FE solution and interpolation affect the fourth decimal digit.

# ROOT ROTATIONS

Table 1

Root rotation compliance coefficients of an orthotropic edge cracked layer with arbitrary generalized end forces acting at a distance  $\gg c_{\min} = h\lambda^{-1/4}$  ( $i = 0, 1, 2$ ) from the crack tip; results apply to any  $a, c \geq c_{\min} = h\lambda^{-1/4}$  with uncertainties lower than  $\pm 2\%$

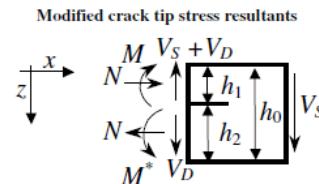
$$\Delta\phi_{0,1} = \frac{1}{E_x h_1} \left( \frac{a_1^M}{h_1} M + a_1^N N + a_1^{Vs} V_S + a_1^{V_D} V_D \right) \quad \Delta\phi_{0,2} = \frac{1}{E_x h_1} \left( \frac{a_2^M}{h_1} M + a_2^N N + a_2^{Vs} V_S + a_2^{V_D} V_D \right)$$

$$a_1^M, a_2^M, a_1^N, a_2^N = b\lambda^{-1/4},$$

$$a_1^{V_D} = b\lambda^{-1/2} - v_{xz}/\kappa_S, \quad a_2^{V_D} = b\lambda^{-1/2} + \eta v_{xz}/\kappa_S$$

$$a_1^{Y_S} = b\lambda^{-1/2} - \frac{1}{1+\eta}\frac{v_{xz}}{\kappa_S}, \quad a_2^{Y_S} = b\lambda^{-1/2} + \frac{\eta}{1+\eta}\frac{v_{xz}}{\kappa_S}$$

with  $\lambda = \frac{E_z}{E_x}$ ,  $\rho = \frac{\sqrt{E_x E_z}}{2G_{xz}}$  and  $\kappa_S = 5/6$



$$M^* = M + N(h_1 + h_2)/2$$

-Root rotations are generated by all crack tip stress resultants and strongly depend on the local two-dimensional fields.

- This result can explain limitations of models based on plate theory and an interface approach in accurately predicting fracture parameters for short and moderately long cracks

### Uncertainties es

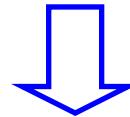
# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

**Energy release rate from 2D elasticity:**

$$\mathcal{G} = \frac{1}{E_x} \sqrt{\frac{1+\rho}{2}} [\lambda^{-3/4} K_I^2 + \lambda^{-1/4} K_{II}^2],$$

**Energy release rate from first order shear deformation beam theory:**

$$\mathcal{G} = J = \frac{1}{2} \left[ \sum_{i=1}^2 \left( \frac{M_i^2}{E_x h_i^3 / 12} + \frac{V_i^2}{\kappa_s G_{xz} h_i} + \frac{N_i^2}{E_x h_i} + 2V_i \Delta \varphi_{0,i} \right) - \frac{M_0^2}{E_x h_0^3 / 12} - \frac{V_0^2}{\kappa_s G_{xz} h_0} - \frac{N_0^2}{E_x h_0} \right],$$



$$f_N(\eta) = \frac{1}{\sqrt{2}} \sqrt{1+4\eta+6\eta^2+3\eta^3}$$

$$f_M(\eta) = \frac{1}{\sqrt{2}} \sqrt{12(1+\eta^3)}$$

$$f_{V_D}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_D} - a_2^{V_D} + \frac{1+\eta}{\kappa_s} \left( \frac{\rho}{\sqrt{\lambda}} + \nu_{xz} \right) \right)}$$

$$f_{V_S}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_S} + \frac{1}{\kappa_s} \frac{1}{1+\eta} \left( \frac{\rho}{\sqrt{\lambda}} + \nu_{xz} \right) \right)}$$

$$\gamma_M(\eta) = \sin^{-1} \left( \frac{3(1+\eta)\eta^2}{f_M f_N} \right)$$

$$\gamma_{V_D}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^N - a_2^N}{2f_N f_{V_D}} \right)$$

$$\gamma_{V_S}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^N}{2f_N f_{V_S}} \right)$$

$$\omega = 52.1^\circ - 3^\circ \eta.$$

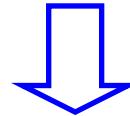
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**Energy release rate from first order shear deformation beam theory:**

$$\mathcal{G} = J = \frac{1}{2} \left[ \sum_{i=1}^2 \left( \frac{M_i^2}{E_x h_i^3 / 12} + \frac{V_i^2}{\kappa_s G_{xz} h_i} + \frac{N_i^2}{E_x h_i} + 2V_i \Delta \varphi_{0,i} \right) - \frac{M_0^2}{E_x h_0^3 / 12} - \frac{V_0^2}{\kappa_s G_{xz} h_0} - \frac{N_0^2}{E_x h_0} \right],$$



$$f_N(\eta) = \frac{1}{\sqrt{2}} \sqrt{1+4\eta+6\eta^2+3\eta^3}$$

$$f_M(\eta) = \frac{1}{\sqrt{2}} \sqrt{12(1+\eta^3)}$$

$$f_{V_D}(\eta, \lambda, \rho) = \sqrt{v_{xw} v_{xz} [1 + \eta (\rho - 1)]}$$

$$\gamma_M(\eta) = \sin^{-1} \left( \frac{3(1+\eta)\eta^2}{f_M f_N} \right)$$

$$\gamma_{V_D}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^N - a_2^N}{2f_N f_{V_D}} \right)$$

-Expression for the energy release rate fully resolve all crack problems where conditions are either mode I or mode II (e.g. laboratory specimens)

$$f_{V_S}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_S} + \frac{1}{\kappa_s} \frac{1}{1+\eta} \left( \frac{\rho}{\sqrt{\lambda}} + \nu_{xz} \right) \right)}$$

# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

## Stress Intensity Factors:

$$K_I = \frac{\lambda^{3/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ \frac{f_M M \sin(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \cos(\omega)}{\sqrt{h_1}} + \frac{f_{V_D} V_D \sin(\gamma_{V_D} + \omega)}{\sqrt{h_1}} + \frac{f_{V_S} V_S \sin(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right],$$

$$K_{II} = \frac{\lambda^{1/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ -\frac{f_M M \cos(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \sin(\omega)}{\sqrt{h_1}} - \frac{f_{V_D} V_D \cos(\gamma_{V_D} + \omega)}{\sqrt{h_1}} - \frac{f_{V_S} V_S \cos(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right].$$

## Semi-analytically derived constants:

$$f_N(\eta) = \frac{1}{\sqrt{2}} \sqrt{1 + 4\eta + 6\eta^2 + 3\eta^3}$$

$$f_M(\eta) = \frac{1}{\sqrt{2}} \sqrt{12(1 + \eta^3)}$$

$$f_{V_D}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_D} - a_2^{V_D} \right) + \frac{1+\eta}{\kappa_s} \left( \frac{\rho}{\sqrt{\lambda}} + \nu_{xz} \right)}$$

$$f_{V_S}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_S} \right) + \frac{1}{\kappa_s} \frac{1}{1+\eta} \left( \frac{\rho}{\sqrt{\lambda}} + \nu_{xz} \right)}$$

$$\gamma_M(\eta) = \sin^{-1} \left( \frac{3(1+\eta)\eta^2}{f_M f_N} \right)$$

$$\gamma_{V_D}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^{V_D} - a_2^{V_D}}{2f_N f_{V_D}} \right)$$

$$\gamma_{V_S}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^{V_S}}{2f_N f_{V_S}} \right)$$

$$\omega = 52.1^\circ - 3^\circ \eta.$$

# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

## Stress Intensity Factors:

$$K_I = \frac{\lambda^{3/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ \frac{f_M M \sin(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \cos(\omega)}{\sqrt{h_1}} + \frac{f_{V_D} V_D \sin(\gamma_{V_D} + \omega)}{\sqrt{h_1}} + \frac{f_{V_S} V_S \sin(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right],$$
$$K_{II} = \frac{\lambda^{1/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ -\frac{f_M M \cos(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \sin(\omega)}{\sqrt{h_1}} - \frac{f_{V_D} V_D \cos(\gamma_{V_D} + \omega)}{\sqrt{h_1}} - \frac{f_{V_S} V_S \cos(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right].$$

## Semi-analytically derived constants:

$$f_N(\eta) = \frac{1}{\sqrt{2}} \sqrt{1 + 4\eta + 6\eta^2 + 3\eta^3}$$

$$\gamma_M(\eta) = \sin^{-1} \left( \frac{3(1+\eta)\eta^2}{f_M f_N} \right)$$

$$f_M(\eta) = \frac{1}{\sqrt{2}}$$

-Expressions for the stress intensity factors fully resolve mixed mode fracture problems in statically determined systems. In statically undeterminate systems calculation of the crack tip stress resultants is first required (using 2D analyses or beam theory models accounting for the root rotations).

$$f_{V_D}(\eta, \lambda, \rho) =$$

-Expressions are very accurate (uncertainties below 2%) for crack lengths

$$f_{V_S}(\eta, \lambda, \rho) = a, c \geq c_{min} = h_i \lambda^{-1/4} \quad (i=0,1,2)$$

# ENERGY RELEASE RATE AND STRESS INTENSITY FACTORS

Table 2

Energy release rates and stress intensity factors in an edge-cracked orthotropic layer subject to arbitrary generalized end forces acting at a distance  $\geq c_{\min} = h_i \lambda^{-1/4}$  ( $i = 0, 1, 2$ ) from the crack tip

*Energy release rate:*

$$\mathcal{G} = J = \frac{1}{2} \left[ \sum_{i=1}^2 \left( \frac{M_i^2}{E_x h_i^3 / 12} + \frac{V_i^2}{\kappa_S G_{xz} h_i} + \frac{N_i^2}{E_x h_i} + 2V_i \Delta \varphi_{0,i} \right) - \frac{M_0^2}{E_x h_0^3 / 12} - \frac{V_0^2}{\kappa_S G_{xz} h_0} - \frac{N_0^2}{E_x h_0} \right]$$

where:

$N_i, M_i, V_i$  ( $i = 1, 2, 3$ ) = crack tip stress resultants (see picture a),

$$\Delta \varphi_{0,i} = \frac{1}{E_x h_i} \left( \frac{a_i^M}{h_i} M + a_i^N N + a_i^{V_s} V_s + a_i^{V_D} V_D \right) = \text{root rotation sub-layer } i (i = 1, 2)$$

$N, M, V_s, V_D$  = modified crack tip stress resultants (see picture b),

$a_i^M, a_i^N, a_i^{V_s}, a_i^{V_D}$  = root rotation compliances ( $i = 1, 2$ ) (Table 1),

$E_x, E_z, G_{xz}, v_{xz}$  = Young's and shear moduli, Poisson ratio.

$$\kappa_S = 5/6$$

*Stress intensity factors:*

$$K_I = \frac{\lambda^{3/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ \frac{f_M M \sin(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \cos(\omega)}{\sqrt{h_1}} + \frac{f_{V_D} V_D \sin(\gamma_{V_D} + \omega)}{\sqrt{h_1}} + \frac{f_{V_S} V_S \sin(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right]$$

$$K_{II} = \frac{\lambda^{1/8}}{\left(\frac{1+\rho}{2}\right)^{1/4}} \left[ -\frac{f_M M \cos(\gamma_M + \omega)}{h_1^{3/2}} + \frac{f_N N \sin(\omega)}{\sqrt{h_1}} - \frac{f_{V_D} V_D \cos(\gamma_{V_D} + \omega)}{\sqrt{h_1}} - \frac{f_{V_S} V_S \cos(\gamma_{V_S} + \omega)}{\sqrt{h_1}} \right]$$

with:

$$f_M(\eta) = \frac{1}{\sqrt{2}} \sqrt{12(1 + \eta^3)}, \quad f_N(\eta) = \frac{1}{\sqrt{2}} \sqrt{1 + 4\eta + 6\eta^2 + 3\eta^3},$$

$$f_{V_S}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_s} + \frac{1}{\kappa_S} \frac{1}{1+\eta} \left( \frac{\rho}{\sqrt{\lambda}} + v_{xz} \right) \right)},$$

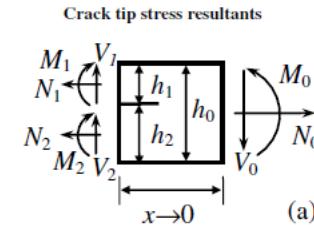
$$f_{V_D}(\eta, \lambda, \rho) = \sqrt{\left( a_1^{V_D} - a_2^{V_D} + \frac{1+\eta}{\kappa_S} \left( \frac{\rho}{\sqrt{\lambda}} + v_{xz} \right) \right)},$$

$$\gamma_M(\eta) = \sin^{-1} \left( \frac{3(1+\eta)\eta^2}{f_M f_N} \right),$$

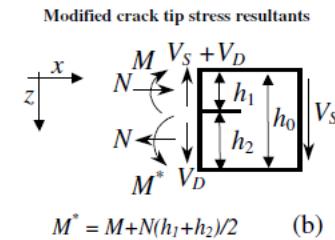
$$\gamma_{V_S}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^N}{2f_N f_{V_S}} \right), \quad \gamma_{V_D}(\eta, \rho) = \sin^{-1} \left( \frac{a_1^N - a_2^N}{2f_N f_{V_D}} \right),$$

$$\omega = 52.1^\circ - 3^\circ \eta$$

$$\lambda = \frac{E_z}{E_x} \text{ and } \rho = \frac{\sqrt{E_z E_x}}{2G_{xz}} - \sqrt{v_{xz} v_{zx}}$$



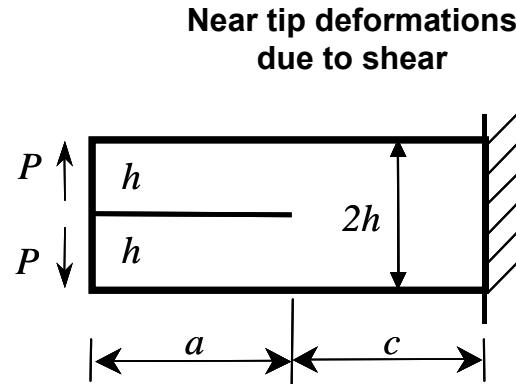
$$\begin{aligned} \text{where:} \\ h_0 &= h_1 + h_2 \\ \eta &= h_1/h \\ M_0 &= M_1 + M_2 + \frac{1}{2}(h_1 N_2 - h_2 N_1), \\ N_0 &= N_1 + N_2, \\ V_0 &= V_1 + V_2. \end{aligned}$$



$$M^* = M + N(h_1 + h_2)/2 \quad (b)$$

$$\begin{aligned} \text{where:} \\ M &= M_1 - \frac{1}{(1+1/\eta)^3} M_0, \\ N &= -N_1 + \frac{1}{1+1/\eta} N_0 - \frac{6}{\eta} \frac{1}{(1+1/\eta)^3} \frac{M_0}{h_1}, \\ V_S &= V_0, \quad V_D = -V_2 \end{aligned}$$

## APPLICATION – The exemplary case of the DCB specimen



Dimensionless energy release rate:

$$\frac{\mathcal{G}_{\text{DCB}} E_x h}{P^2} = 12 \left( \frac{a}{h} \right)^2 \left[ 1 + \frac{1}{6} \left( \frac{1}{\kappa_S} \left( \frac{\rho}{\sqrt{\lambda}} + v_{xz} \right) + \frac{a_1^{V_D} - a_2^{V_D}}{2} \right) \left( \frac{h}{a} \right)^2 + \frac{1}{12} (a_1^M - a_2^M) \left( \frac{h}{a} \right) \right],$$

Elementary beam theory

Shear deformations along the arms

Near tip deformations due to shear

Near tip deformations due to bending (and shear)

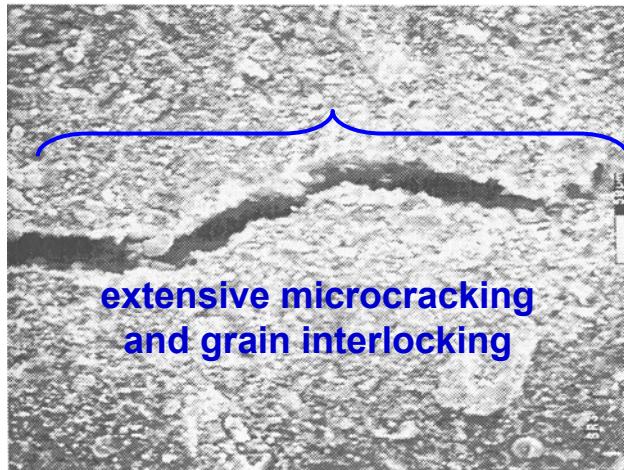
Dimensionless energy release rate for degenerate orthotropic beams:

$$\frac{\mathcal{G}_{\text{DCB}} E_x h}{P^2} = 12 \left( \frac{a}{h} \right)^2 \left[ 1 + 0.673 \lambda^{-1/4} \left( \frac{h}{a} \right) \right]^2.$$

## BRIDGED- AND COHESIVE-CRACK MODELS

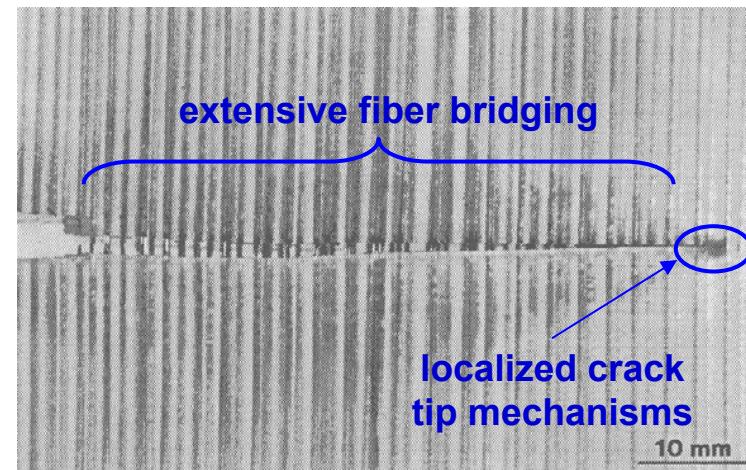
To describe fracture processes where nonlinear mechanisms arise in narrow, finite size bands, or process zones, ahead of traction free cracks

Cohesive crack



epoxy mortar  
(Steiger, Sadouki & Wittmann)

Bridged crack

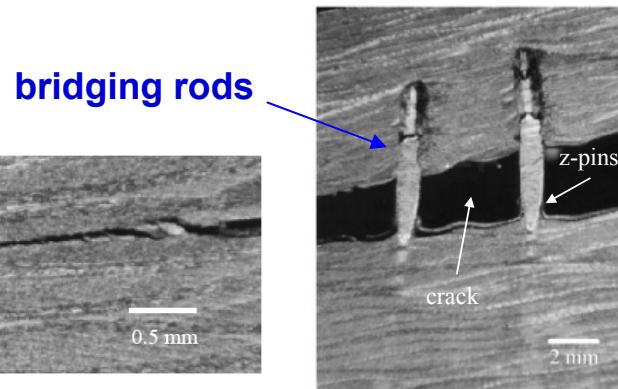


PMMA/Al composite  
(Zok & Hom, 1990)

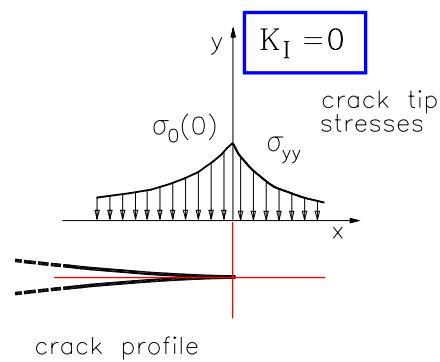
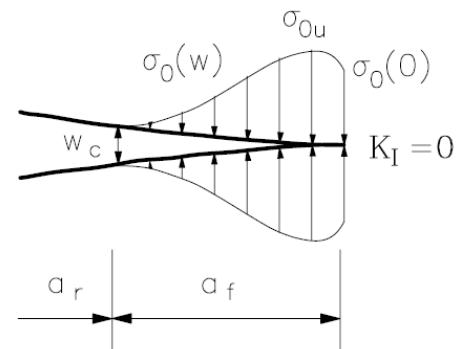
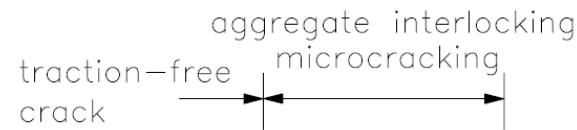
crack tip  
microcracks



carbon-epoxy laminate/Ti z-pins  
(Rugg, Cox & Massabò, 2002)

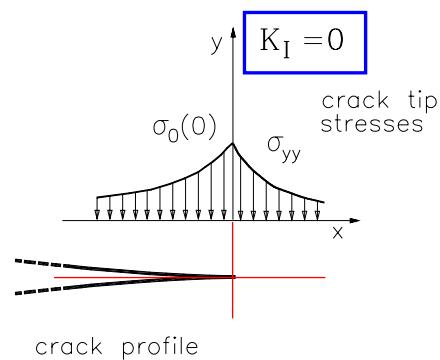
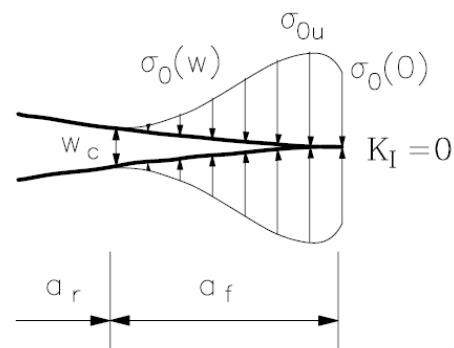


## COHESIVE CRACK MODEL



## COHESIVE CRACK MODEL

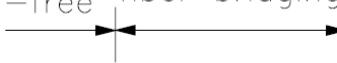
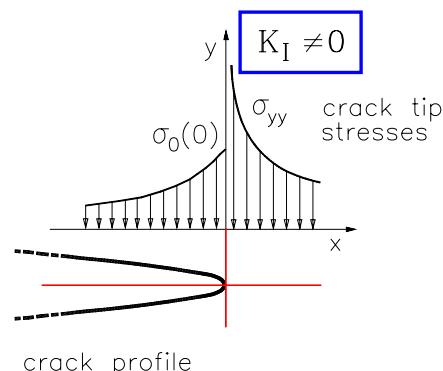
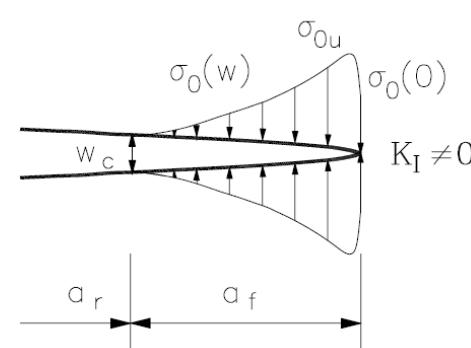
aggregate interlocking  
traction-free crack

## BRIDGED CRACK MODEL

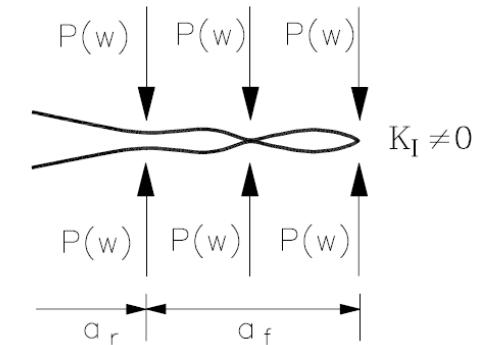
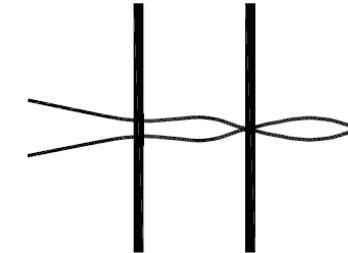
### continuous model

traction-free fiber bridging  
crack

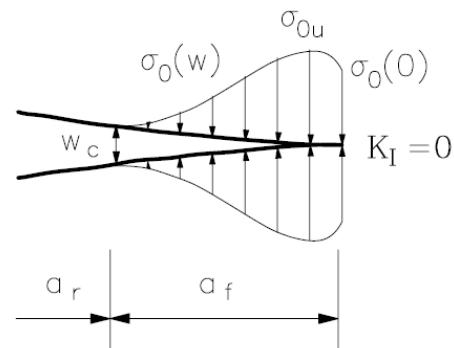
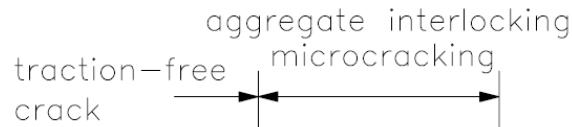



### discontinuous model

traction-free fiber bridging  
crack

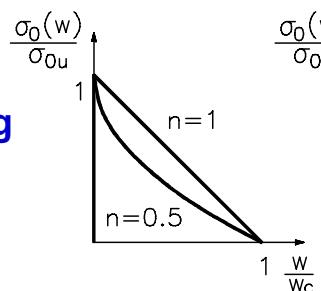



# COHESIVE CRACK MODEL



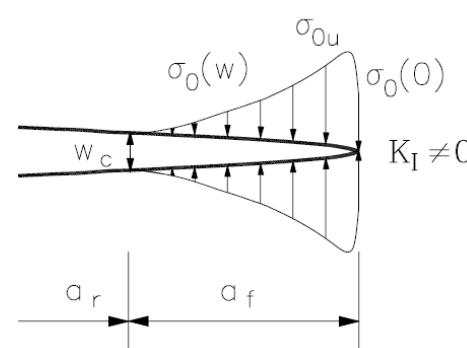
## Model parameters:

## cohesive/bridging traction laws

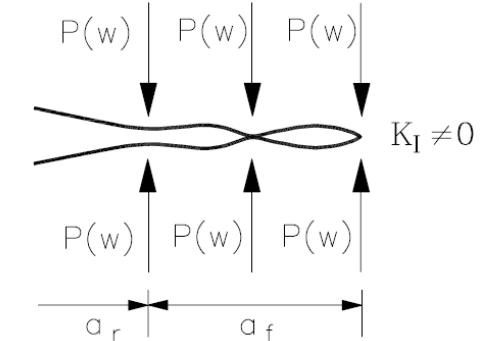
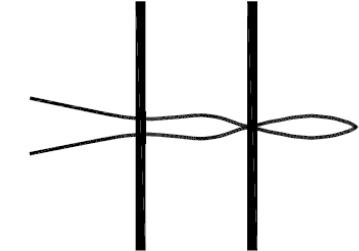


$$\frac{\sigma_0(w)}{\sigma_{0u}} = 1 - \left( \frac{w}{w_c} \right)^n$$

## continuous model

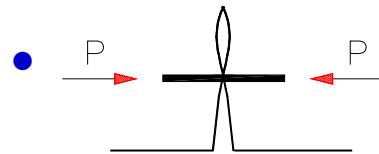
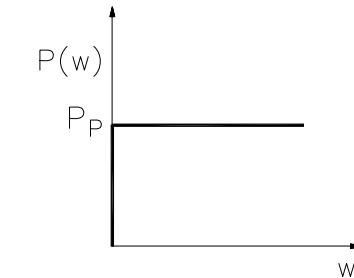
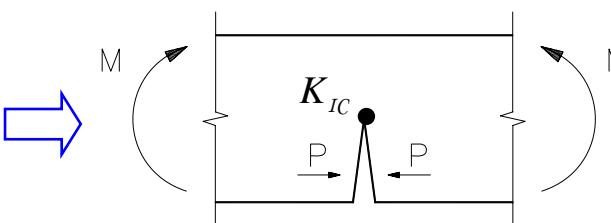
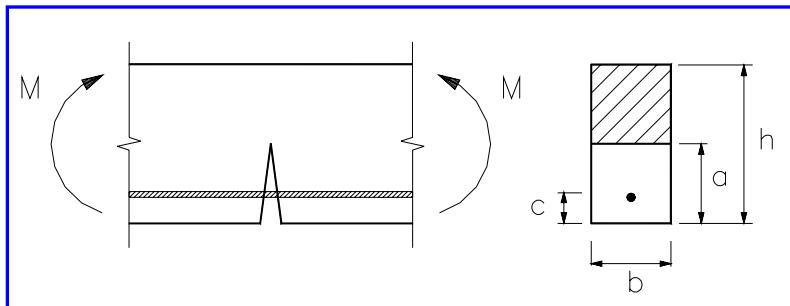


**discontinuous model**



$G_{\text{IC}}$  or  $K_{\text{IC}}$   
n bridged crack  
model)

## BRIDGED CRACK MODEL - SOLUTION



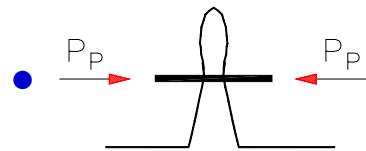
**statically indeterminate problem**

**Kinematic compatibility condition → determination of forces P**

$$w = w_M + w_P = \frac{2M}{E} \int_c^a \frac{K_{IP} K_{IM}}{PM} b da + \frac{2P}{E} \int_c^a \frac{K_{IP}^2}{P^2} b da = 0$$

$w_M$

$w_P$   
(Castigliano's theorem)



$$w = f(M, P_P, \dots)$$

**Crack propagation:**

$$K_I = K_{IM} - K_{IP} = K_{IC}$$

(Carpinteri, 1981, 1984; Bosco & Carpinteri, 1992; Carpinteri & Massabò, 1996, 1997)

**Extension to other problems: weight function method**

(e.g. weight functions for orthotropic double cantilever beams in Brandinelli, Massabò & Cox, 2003; Brandinelli & Massabò, 2006)

# DEVELOPMENT OF BRIDGED-CRACK MODELS

Barenblatt (1959)

Material systems

Dugdale (1960)

crystals

( $K_I = 0$ )

Bilby, Cottrell & Swinden (1963)

metals

...

Romualdi & Batson (1963)

reinforced concrete

Carpinteri (1981, 1984)

reinforced concrete

Marshall, Cox & Evans (1985)

fiber reinforced ceramics

Budiansky, Hutchinson & Evans (1986)

fiber reinforced ceramics

Jenq & Shah (1985,1986)

fiber reinforced concrete

Foote, Mai & Cotterell (1986)

fiber reinforced cementitious composites

Rose (1987a,b)

crack reinforcement by springs and patches

Swanson et al. (1987)

coarse grain ceramics

( $K_I \neq 0$ )

Erdogan & Joseph (1987)

particle reinforced ceramics

Mc Meeking & Evans (1990)

metal matrix composites, fatigue

Kendall, Clegg & Gregory (1991)

polymer crazing

Bower & Ortiz (1991)

particle reinforced brittle matrix composites

Suo, Ho & Gong (1993)

ceramic matrix composites, notch sensitivity

Ballarini & Muju (1993)

brittle matrix composites

Carpinteri & Massabò (1996)

fiber reinforced cementitious composites

Massabò & Cox (1999)

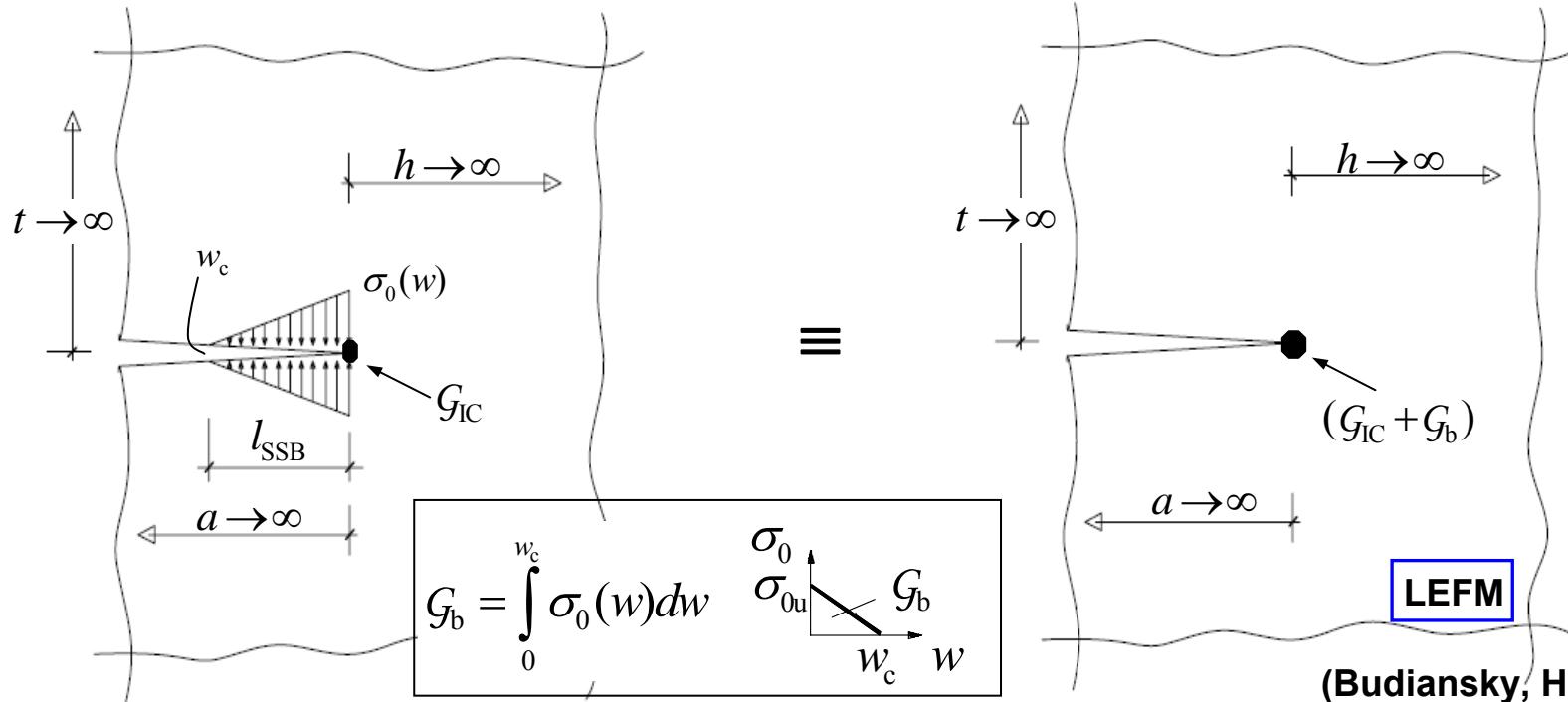
through-thickness reinforced laminates

...

(For reviews: Bao & Suo, 1992; Cox & Marshall, 1994; Massabò, 1999)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS

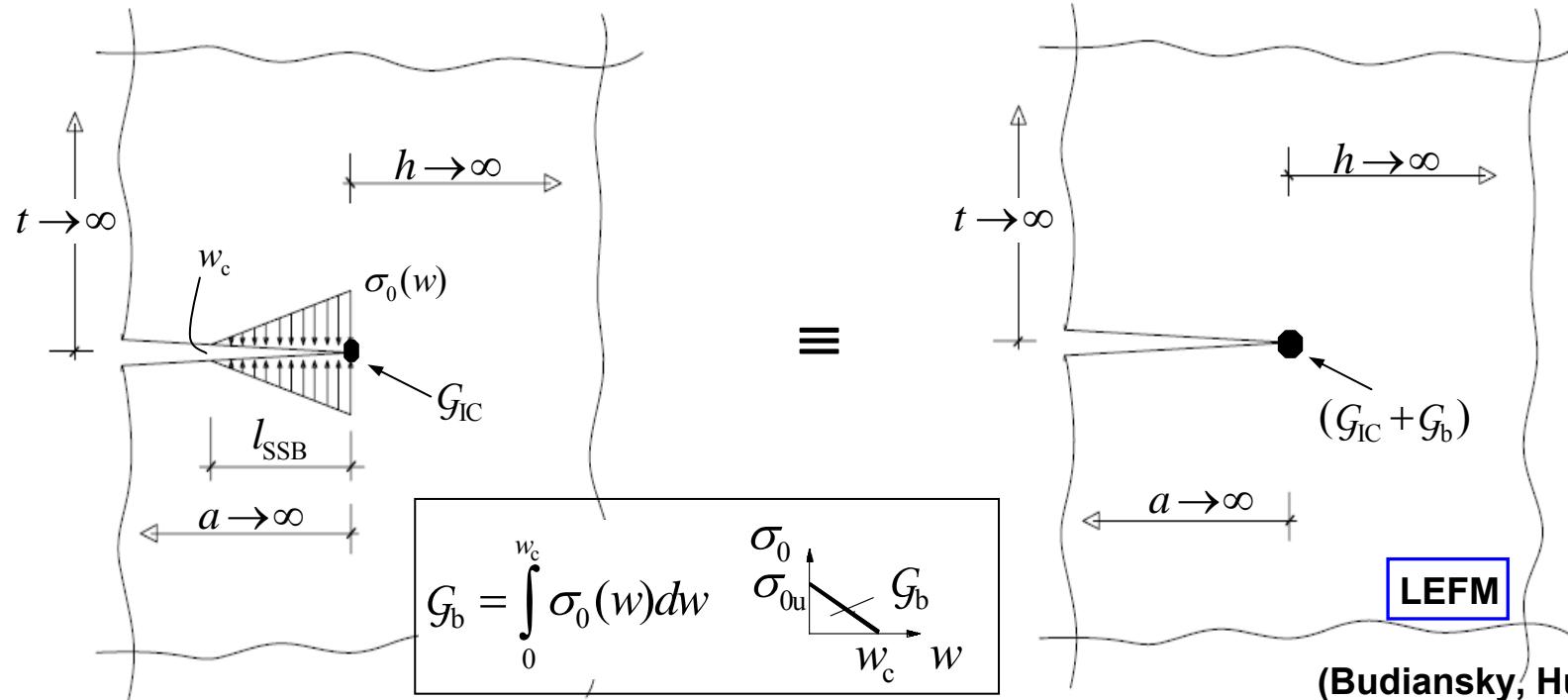
## THE SMALL SCALE BRIDGING LIMIT



(Budiansky, Hutchinson  
& Evans, 1986; Rose, 1987)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS

## THE SMALL SCALE BRIDGING LIMIT



**Small scale bridging characteristic length scales:**

**Bridged crack**  
 $(G_{IC} \neq 0)$

**Cohesive crack**  
 $(G_{IC} = 0)$

$$l_{SSB} = \frac{\pi}{8} \frac{w_c E}{\underbrace{\sigma_{0u}}_{\text{For rectangular bridging law}}} \left( \sqrt{1 + \frac{G_{IC}}{G_b}} - \sqrt{\frac{G_{IC}}{G_b}} \right)^2$$

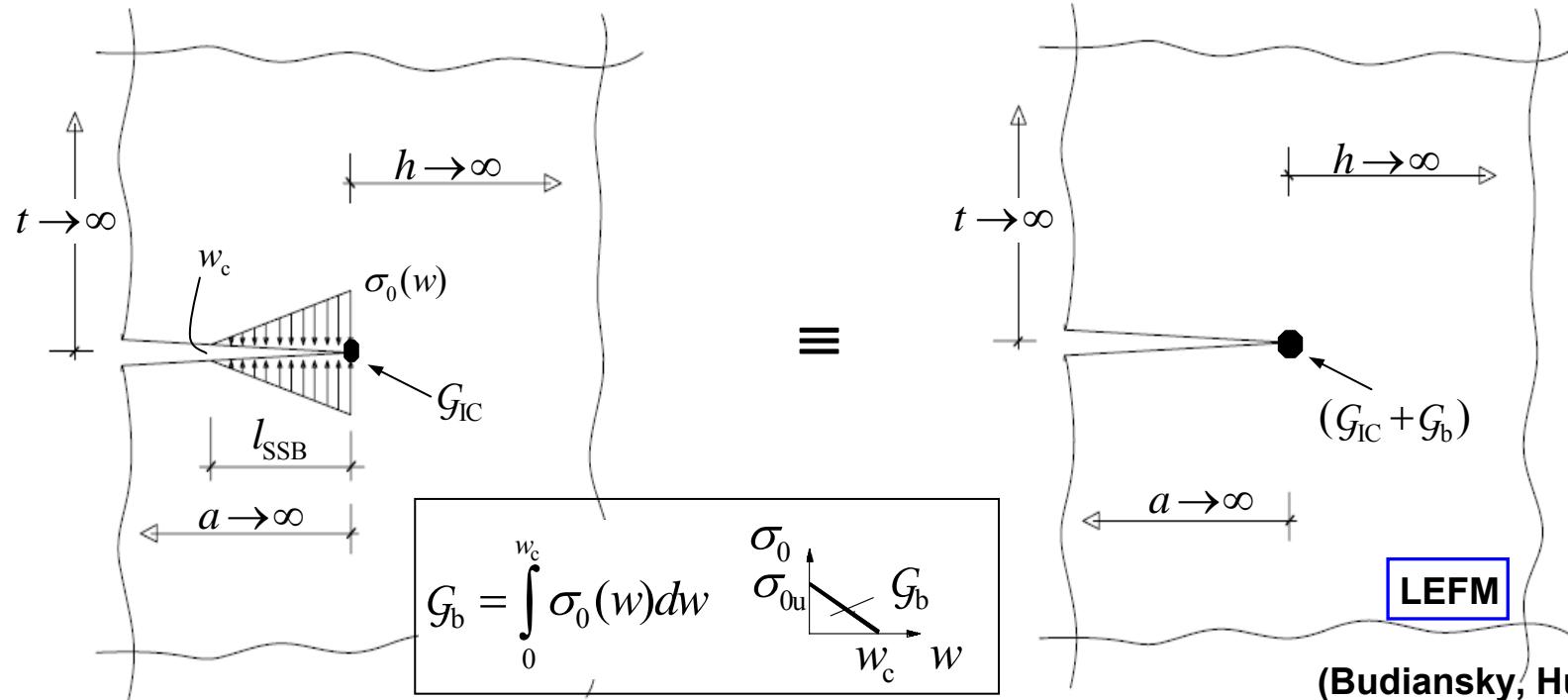
$$l_{SSB} = \frac{\pi}{8} \frac{w_c E}{\sigma_{0u}}$$

**(For rectangular bridging law)**  
(Bao & Suo, 1992; Cox &  
Marshall, 1994)

(Bilby, Cottrell, Swinden, 1963; Hillerborg, 1976)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS

## THE SMALL SCALE BRIDGING LIMIT



(Budiansky, Hutchinson & Evans, 1986; Rose, 1987)

### Small scale bridging characteristic length scales:

**Bridged crack**  
 $(G_{IC} \neq 0)$

**Cohesive crack**  
 $(G_{IC} = 0)$

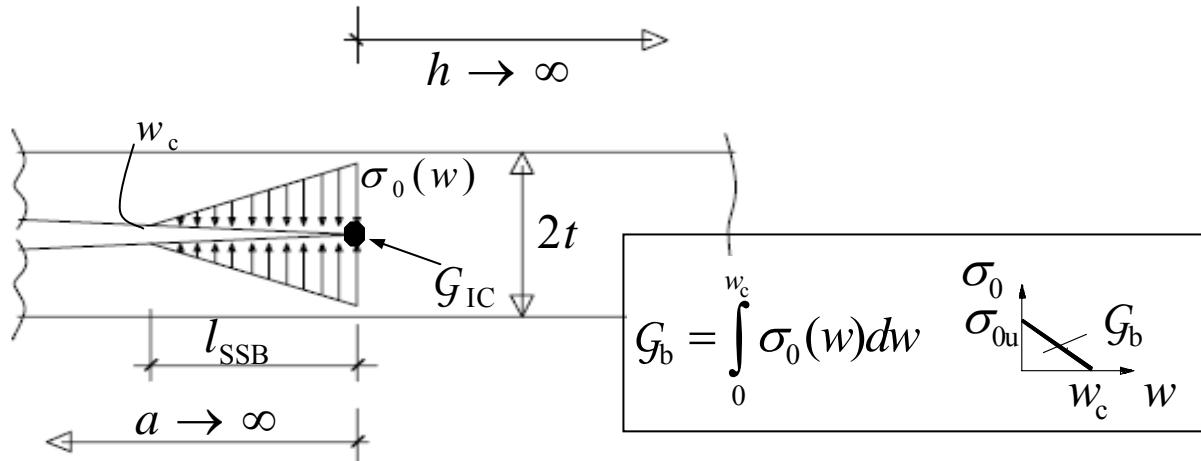
$$l_{SSB} = \frac{\pi w_c E}{\sqrt{G_{IC}}} \left( \frac{\sqrt{G_{IC}}}{\sigma_{0u}} \right)^2$$

Small scale bridging characteristic length scale  
defines material brittleness and varies over many  
order of magnitude in different material systems

$$l_{SSB} = \frac{\pi w_c E}{8 \sigma_{0u}}$$

(Bilby, Cottrell, Swinden, 1963; Hillerborg, 1976)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS THE SMALL SCALE BRIDGING LIMIT IN SLENDER BODIES



**Mode I and mode II characteristic length scales:**

$$l_{SSB}^I \approx (l_{SSB})^{1/4} t^{3/4}$$

(for mode I fracture)

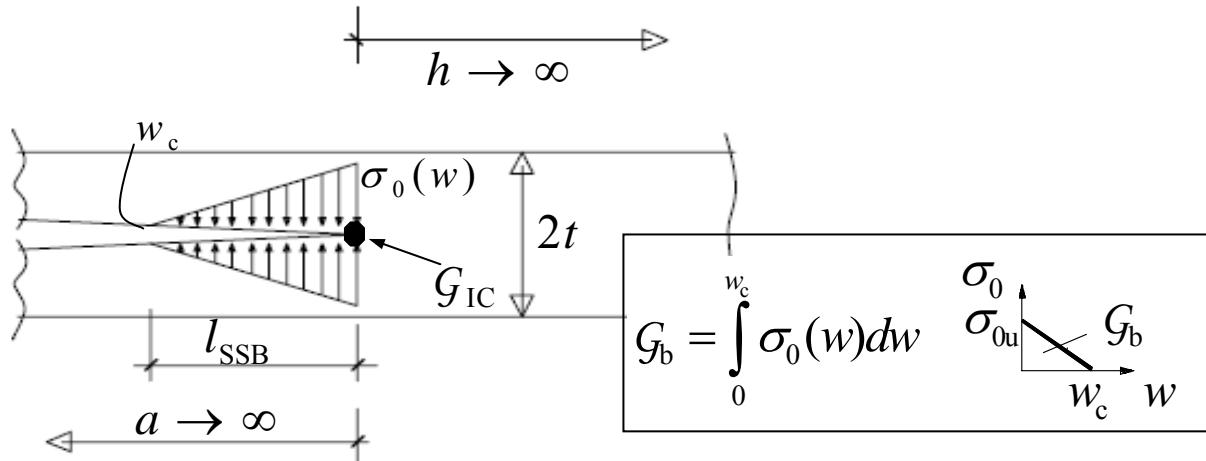
$$l_{SSB}^{II} \approx (l_{SSB} t)^{1/2}$$

(for mode II fracture)

where  $l_{SSB}$  is the characteristic length scale in an infinite body

(Suo, Bao, Fan, 1992;  
Massabo & Cox, 1999)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS THE SMALL SCALE BRIDGING LIMIT IN SLENDER BODIES



## Mode I and mode II characteristic length scales:

I      II      1/4 , 3/4

Characteristic length scales in slender bodies are smaller than those in infinite bodies

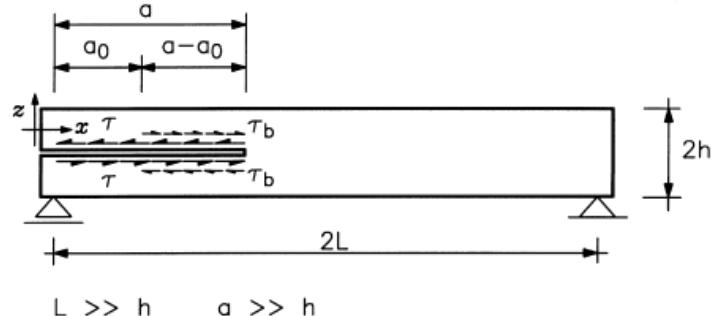
Important consequences when length scale is used to size the mesh in numerical descriptions of fracture processes

where  $l_{SSB}$  is the characteristic length scale in an infinite body

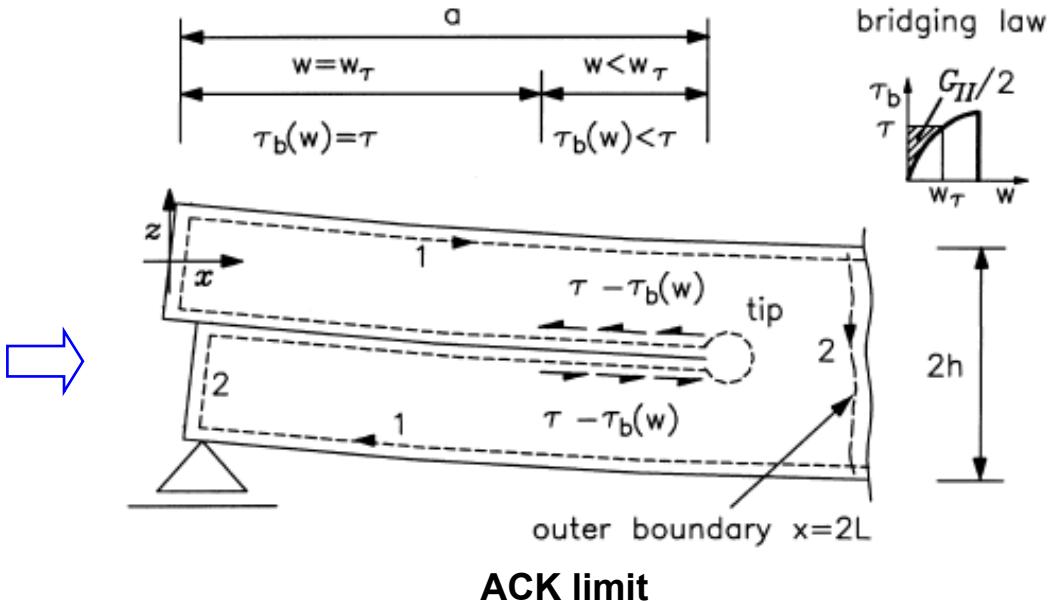
(Suo, Bao, Fan, 1992;  
Massabo & Cox, 1999)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS THE ACK LIMIT IN SLENDER AND NON SLENDER BODIES

(Avenston, Cooper & Kelly, 1971)



Slender body loaded in mode II



ACK characteristic length scales:

$$l_{\text{ACK}} = \frac{\pi E}{4} \left( \frac{1+\alpha}{2\alpha} G_{\text{lc}} \right)^{\frac{1-\alpha}{1+\alpha}} \beta^{\frac{-2}{1+\alpha}}$$

$$l_{\text{ACK}}^{\text{II}} \approx (l_{\text{ACK}} t)^{1/2}$$

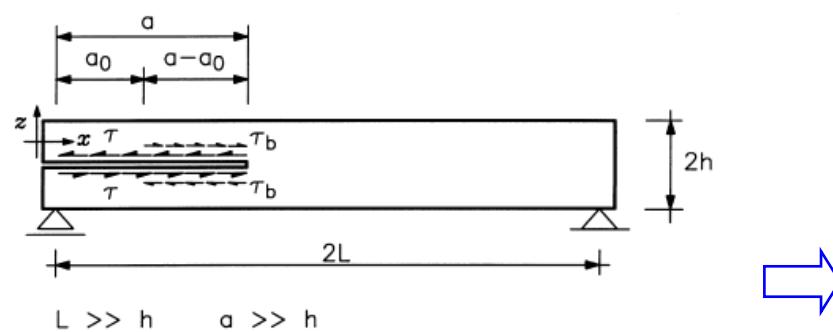
For power law bridging:

$$\sigma_0(w) = \beta (w/2)^\alpha$$

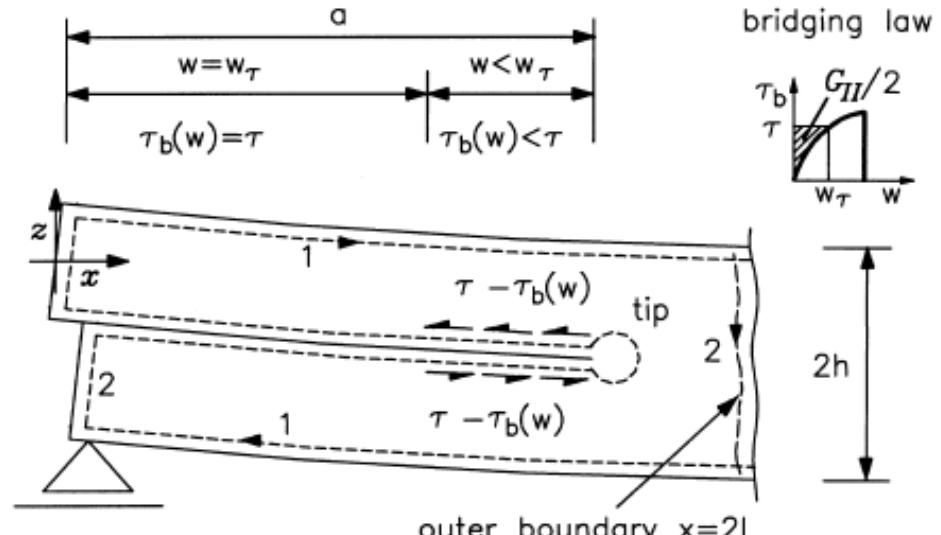
in an infinite body  
(Cox & Marshall, 1994)

in a slender body (mode II)  
(Massabò & Cox, 1999)

# ASYMPTOTIC SOLUTIONS FOR BRIDGED CRACKS THE ACK LIMIT IN SLENDER AND NON SLENDER BODIES



Slender body loaded in mode II



## ACK character

Comparing the characteristic length scales in long unnotched bodies determines whether crack will approach:

- ssb limit (if  $l_{ssb} \ll l_{ACK}$ )  $\rightarrow$  catastrophic failure
- ACK limit (if  $l_{ACK} \ll l_{ssb}$ )  $\rightarrow$  noncatastrophic failure

The presence of a notch favours catastrophic failure

If length scales are similar or body is finite, crack growth is in large scale bridging and detailed calculations are required

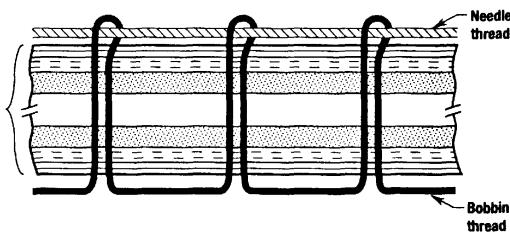
ridging:  
 $(\tau / 2)^\alpha$

dy  
(994)

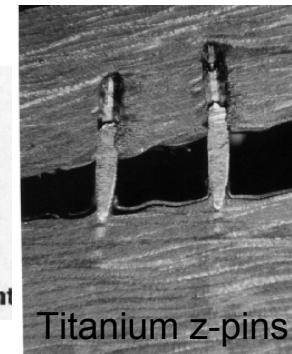
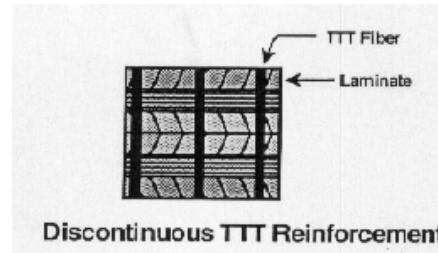
y (mode II)  
(999)

# LARGE SCALE BRIDGING FRACTURE IN THROUGH-THICKNESS REINFORCED LAMINATES

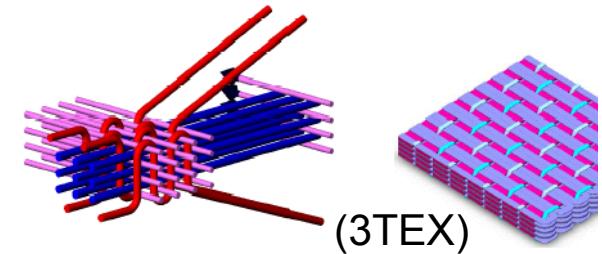
**CONTINUOUS TTR**  
(stitching / weaving)



**DISCONTINUOUS TTR**  
(Fibrous/metallic Z-pins)

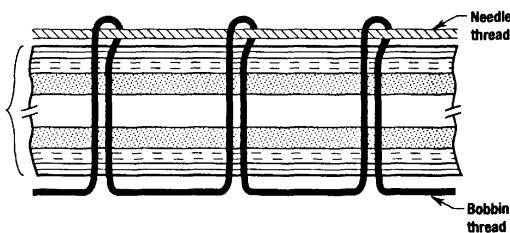


**3D woven composites**

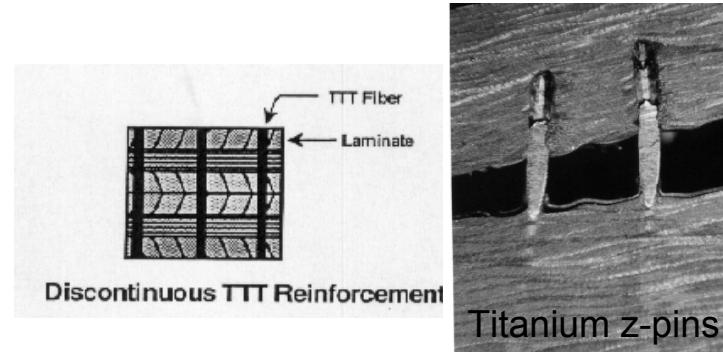


# LARGE SCALE BRIDGING FRACTURE IN THROUGH-THICKNESS REINFORCED LAMINATES

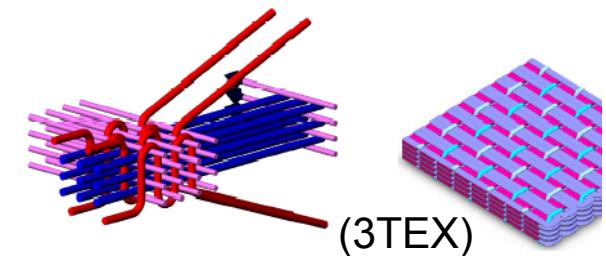
## CONTINUOUS TTR (stitching / weaving)



## DISCONTINUOUS TTR (Fibrous/metallic Z-pins)

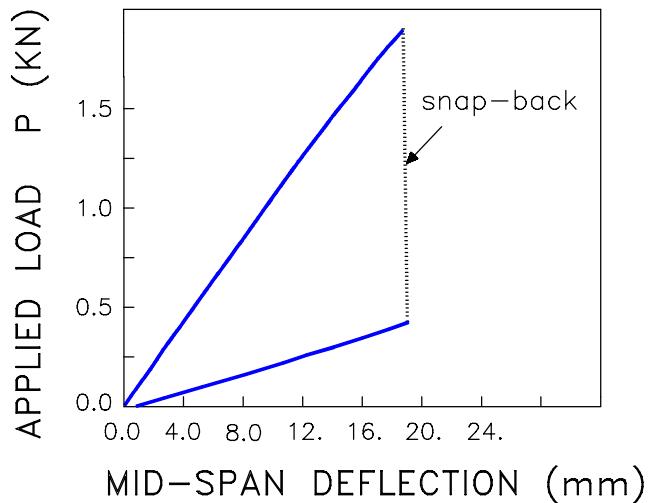


## 3D woven composites



### Load versus mid-span deflection curves in ENF specimens

#### unstitched



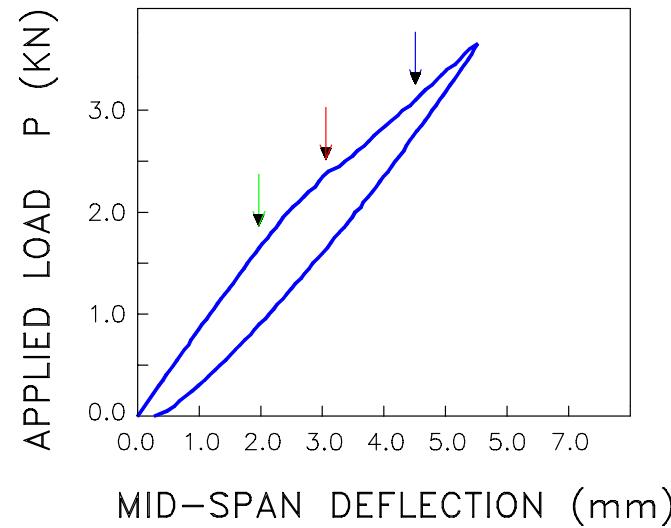
#### Unstitched

Unstitched

$P$

$2L=240$  mm  
 $2h=6.64$  mm  
 $d=23.72$  mm  
 $a_0=27$  mm

#### stitched



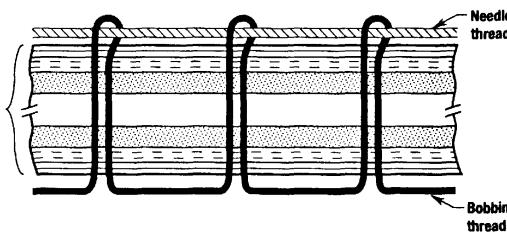
#### Glass stitches

$P$

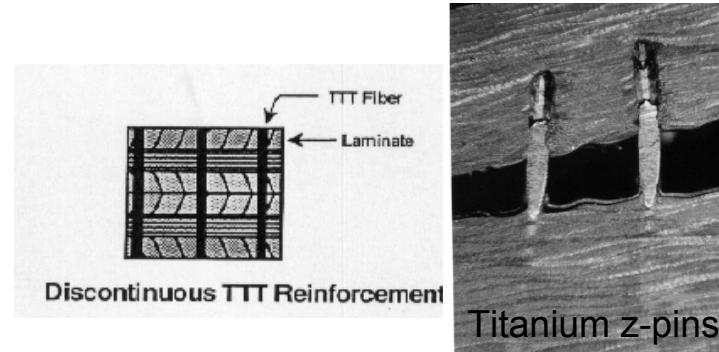
$2L=120$  mm  
 $2h=7.2$  mm  
 $d=24.07$  mm  
 $a_0=20$  mm

# LARGE SCALE BRIDGING FRACTURE IN THROUGH-THICKNESS REINFORCED LAMINATES

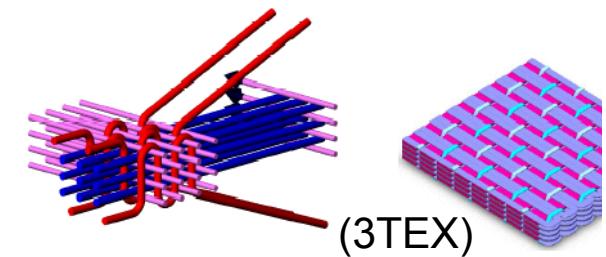
## CONTINUOUS TTR (stitching / weaving)



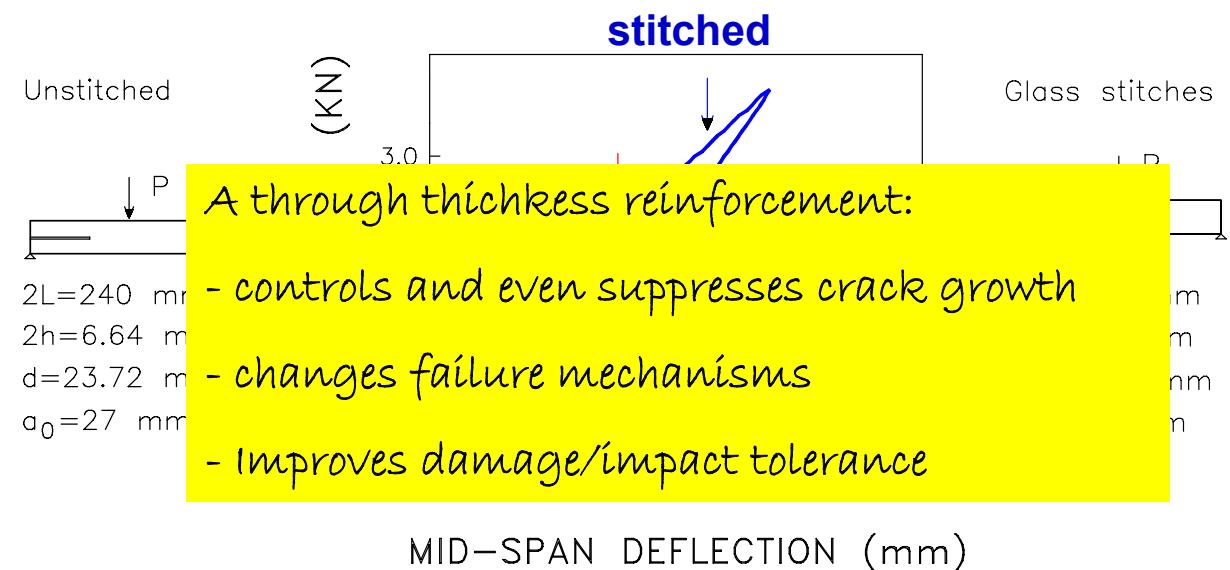
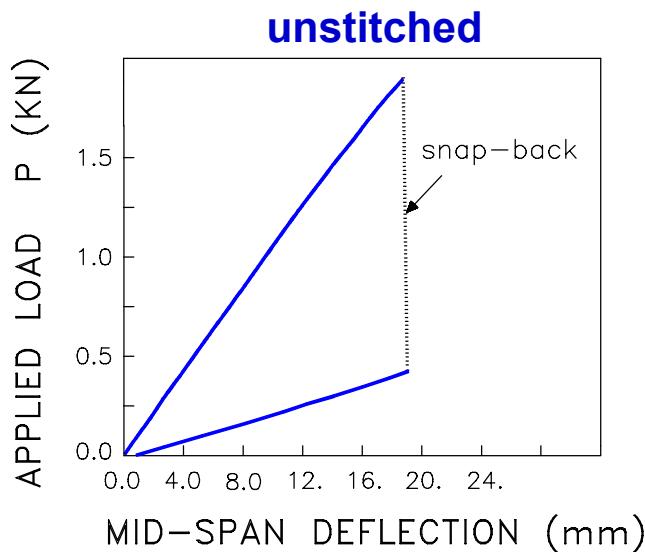
## DISCONTINUOUS TTR (Fibrous/metallic Z-pins)



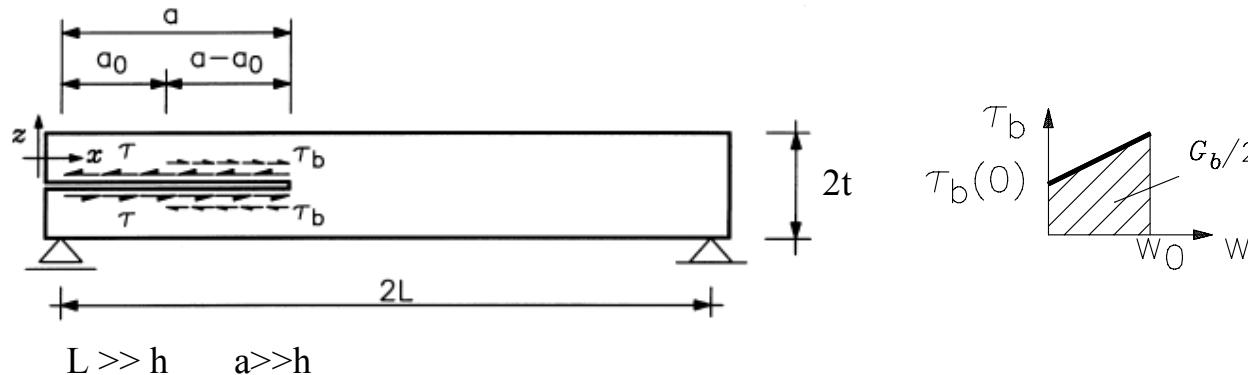
## 3D woven composites



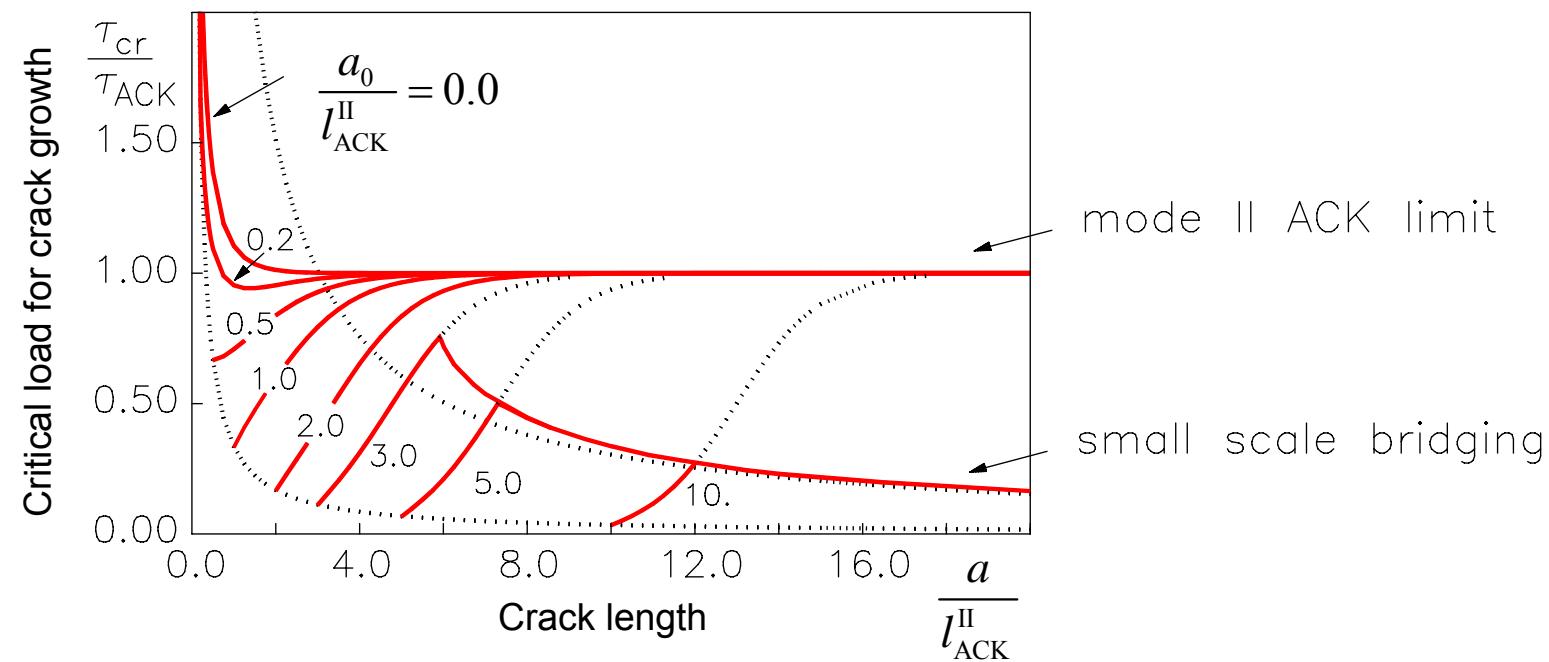
### Load versus mid-span deflection curves in ENF specimens



# TRANSITION FROM NON-CATASTROPHIC TO CATASTROPHIC FAILURE IN SLENDER BODIES



$$\begin{aligned}\tau_b(w) &= \tau_b(0) + \beta w \\ \tau_b(0) &= 2(G_{\text{IIC}}\beta)^{1/2} \\ G_b / G_{\text{IIC}} &= 80\end{aligned}$$

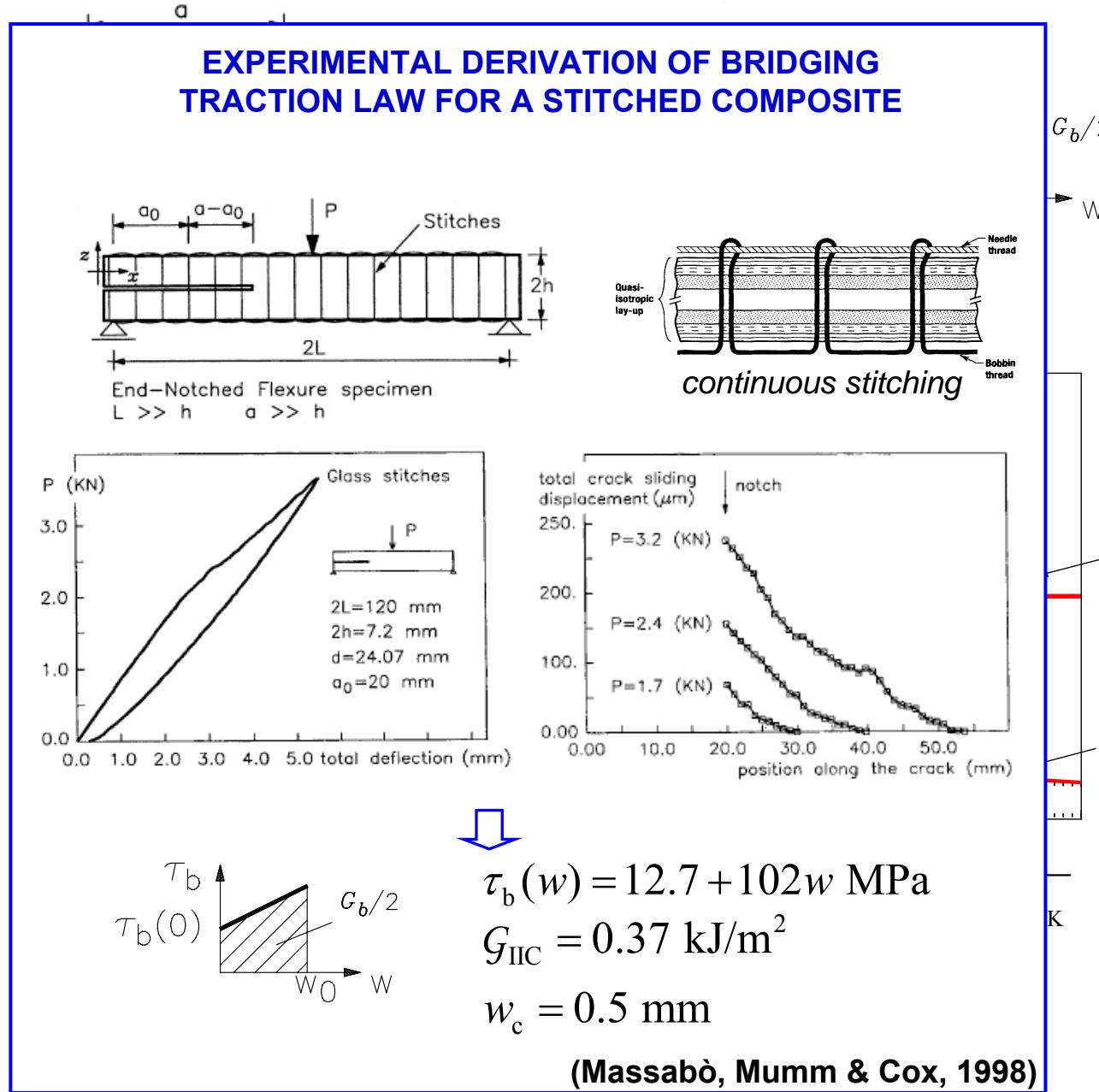


$$\tau_{\text{ACK}} = \tau_b(0) + (G_{\text{IIC}}\beta)^{1/2}$$

$$l_{\text{ACK}}^{\text{II}} = \sqrt{(E/4\beta)t}$$

(Massabò & Cox, 1999)

# TRANSITION FROM NON-CATASTROPHIC TO CATASTROPHIC FAILURE IN SLENDER BODIES



$$\tau_b(w) = \tau_b(0) + \beta w$$

$$\tau_b(0) = 2(G_{\text{IIC}} \beta)^{1/2}$$

$$G_b / G_{\text{IIC}} = 80$$

$G_b/2$

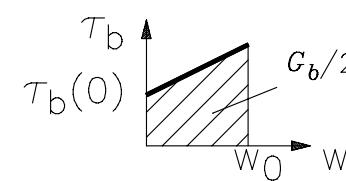
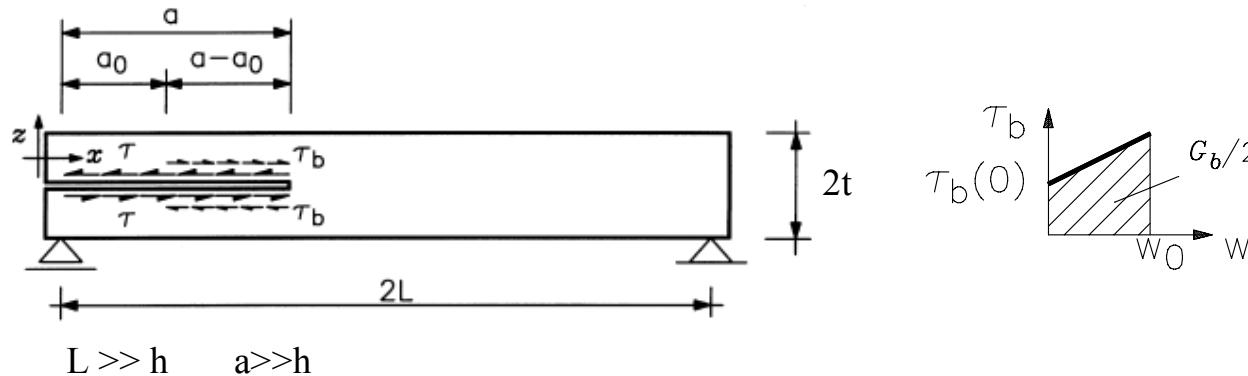
$w$

mode II ACK limit

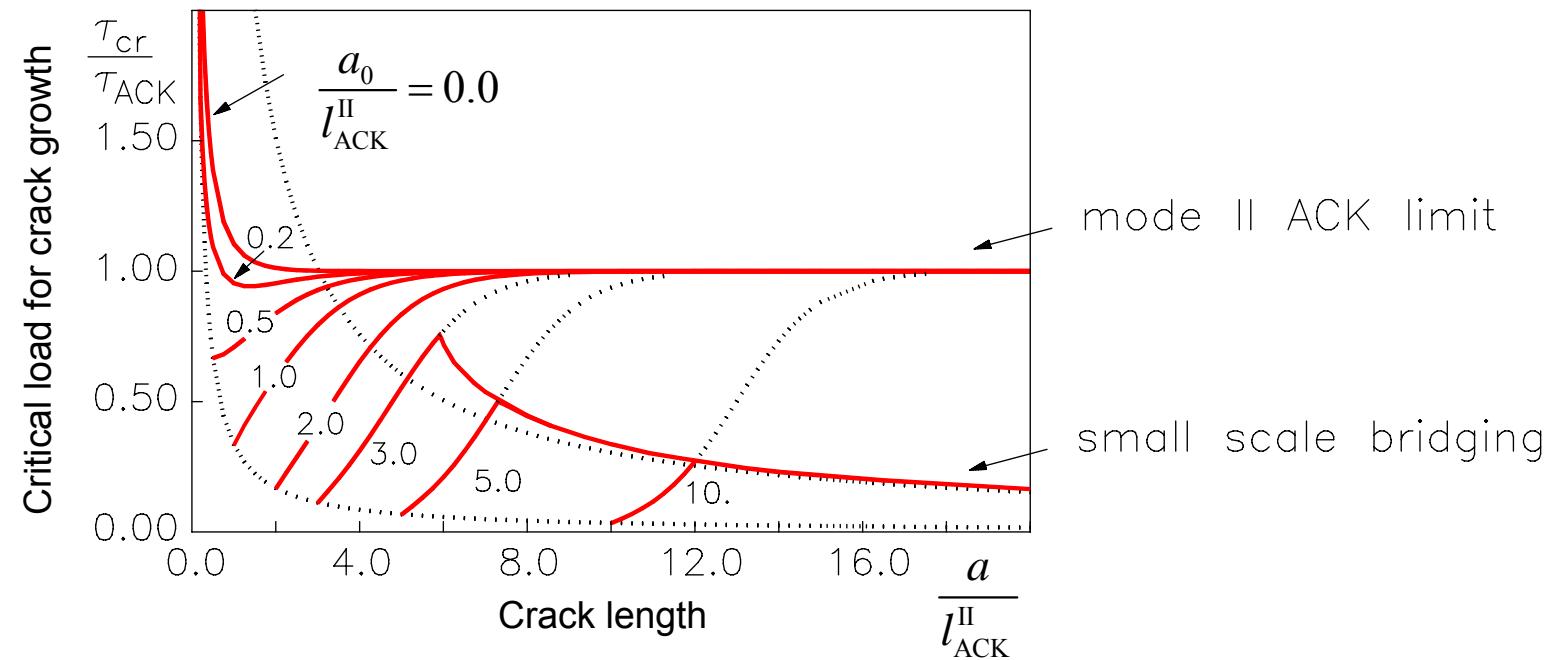
small scale bridging

K

# TRANSITION FROM NON-CATASTROPHIC TO CATASTROPHIC FAILURE IN SLENDER BODIES



$$\begin{aligned}\tau_b(w) &= \tau_b(0) + \beta w \\ \tau_b(0) &= 2(G_{\text{IIC}}\beta)^{1/2} \\ G_b / G_{\text{IIC}} &= 80\end{aligned}$$

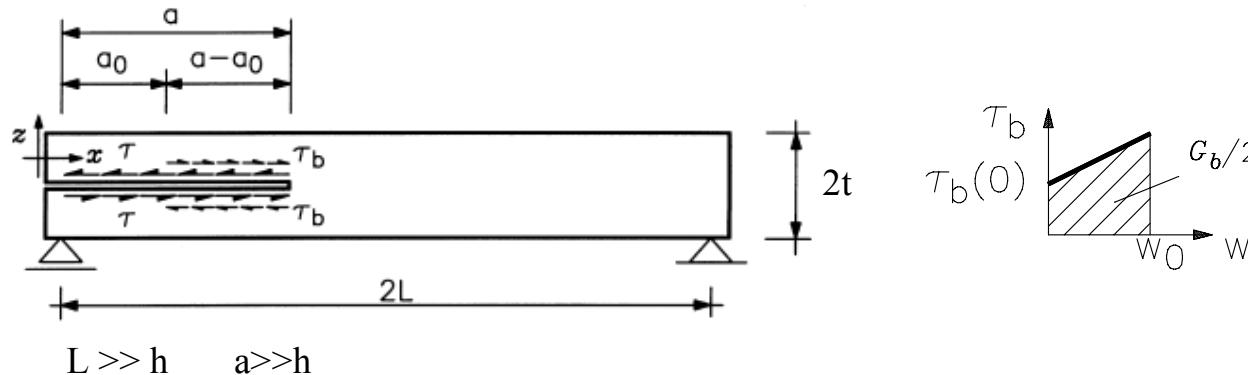


$$\tau_{\text{ACK}} = \tau_b(0) + (G_{\text{IIC}}\beta)^{1/2}$$

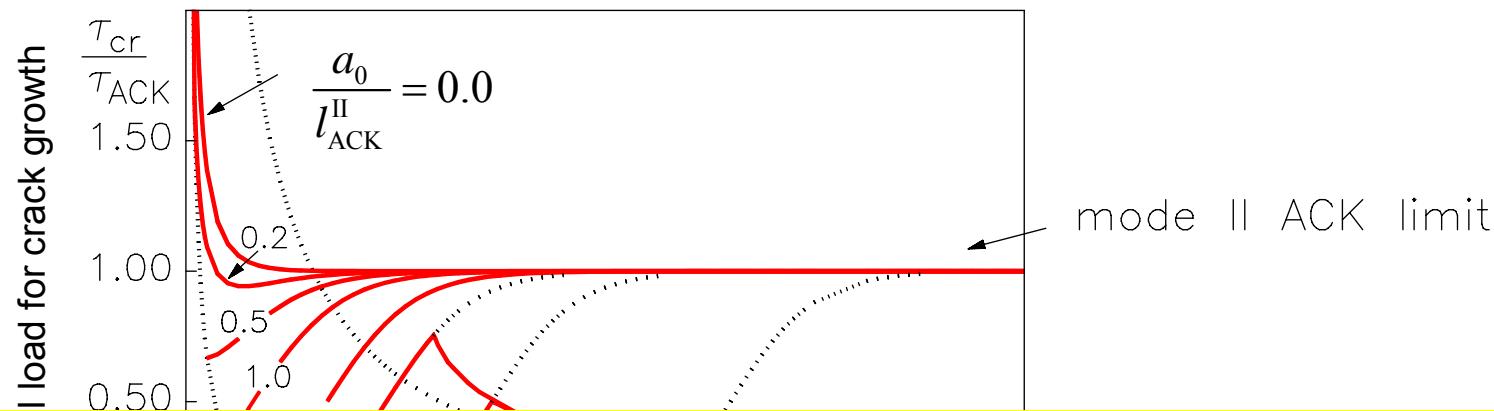
$$l_{\text{ACK}}^{\text{II}} = \sqrt{(E/4\beta)t}$$

(Massabò & Cox, 1999)

# TRANSITION FROM NON-CATASTROPHIC TO CATASTROPHIC FAILURE IN SLENDER BODIES



$$\begin{aligned}\tau_b(w) &= \tau_b(0) + \beta w \\ \tau_b(0) &= 2(G_{\text{IIC}}\beta)^{1/2} \\ G_b / G_{\text{IIC}} &= 80\end{aligned}$$



Design of and with advanced composites:

In through-thickness reinforced laminates overdesigning should be avoided since insertion process degrades inplane properties → bridging action must be understood and quantified using characteristic length scales and bridged-crack modeling

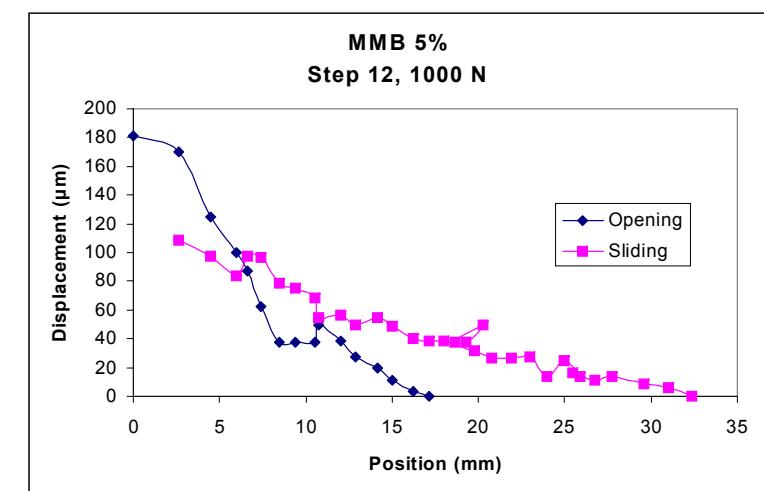
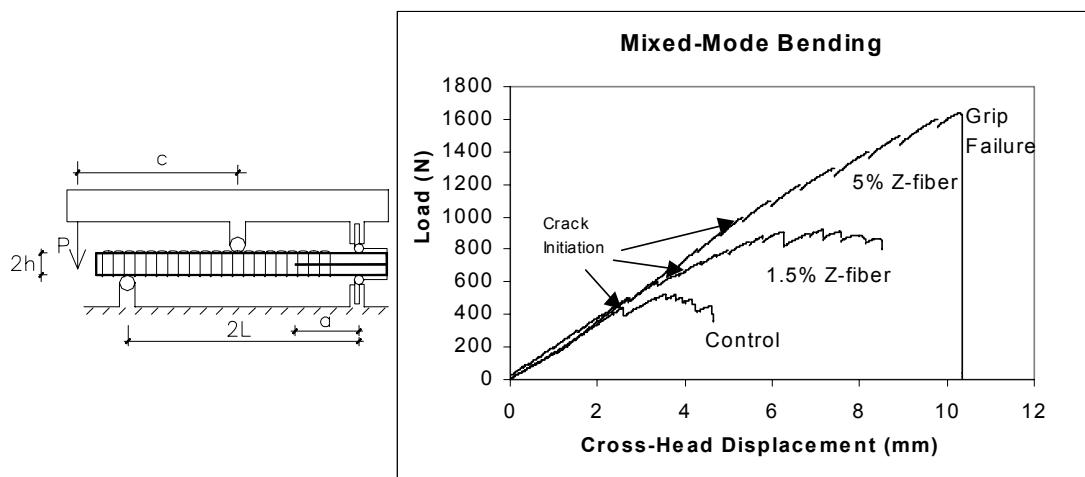
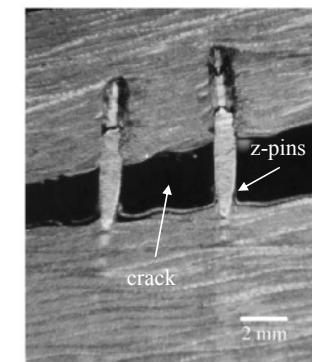
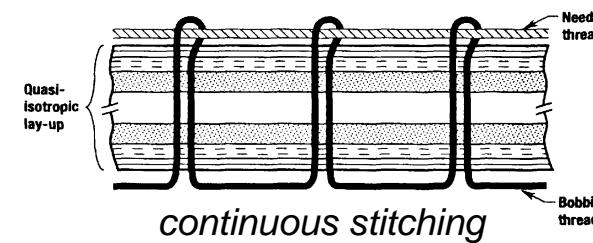
$$\tau_{\text{ACK}} = \tau_b(0) + (G_{\text{IIC}}\beta)^{1/2}$$

$$l_{\text{ACK}}^{\text{II}} = \sqrt{(E/4\beta)t}$$

(Massabò & Cox, 1999)

## OTHER CHARACTERISTIC LENGTH SCALES

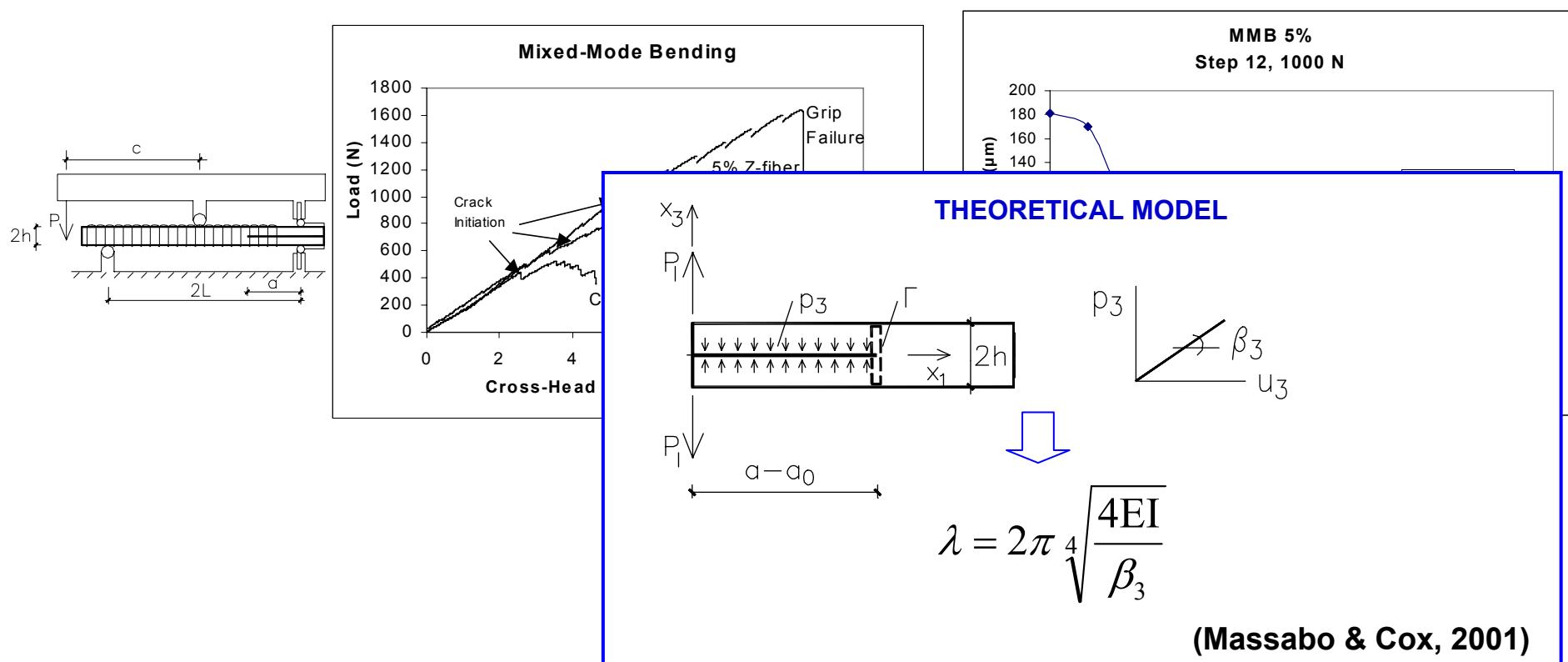
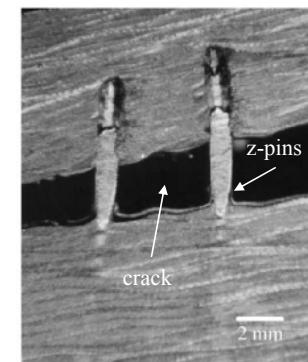
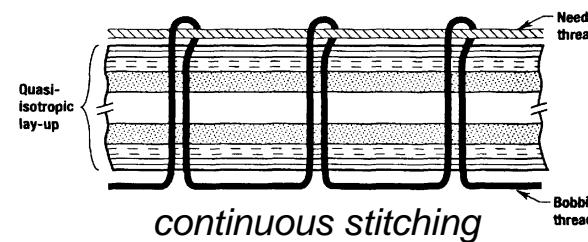
Through thickness reinforced laminates



(Rugg, Cox & Massabo, 2002)

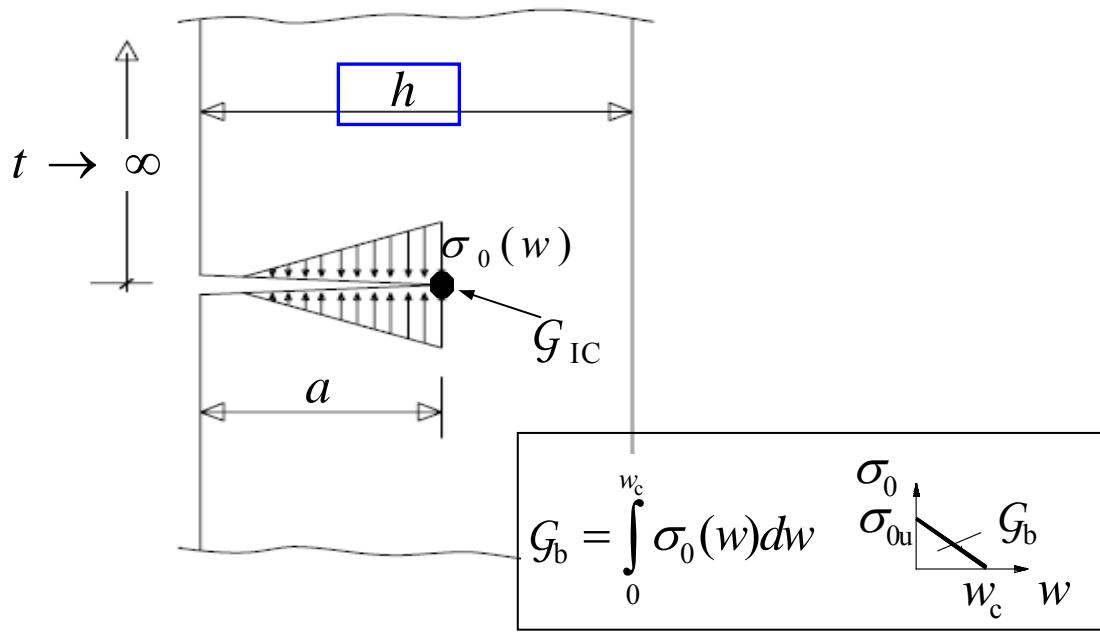
## OTHER CHARACTERISTIC LENGTH SCALES

### Through thickness reinforced laminates



# FRACTURE IN FINITE SIZE BODIES

## DIMENSIONLESS GROUPS GOVERNING MECHANICAL BEHAVIOR



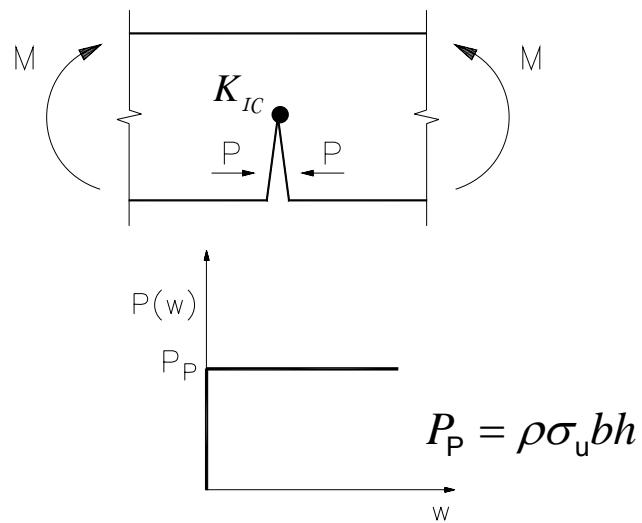
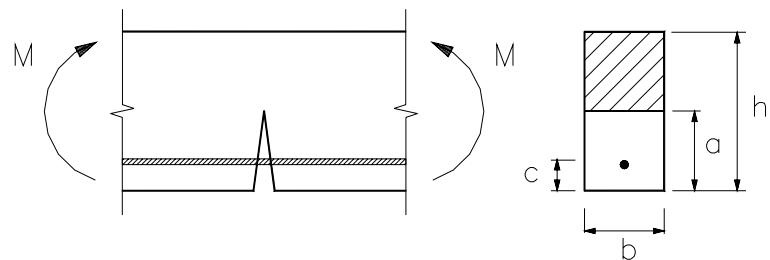
**Cohesive crack:**  
 $(G_{IC} = 0)$

$$\frac{l_{SSB}}{h} \quad (\text{one group})$$

**Bridged crack:**  
 $(G_{IC} \neq 0)$

$$\frac{l_{SSB}}{h} \quad \text{and} \quad \frac{G_{IC}}{G_b} \quad (\text{two groups})$$

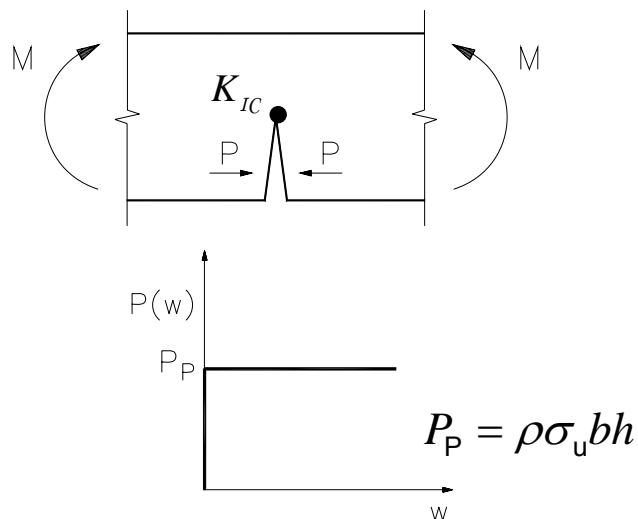
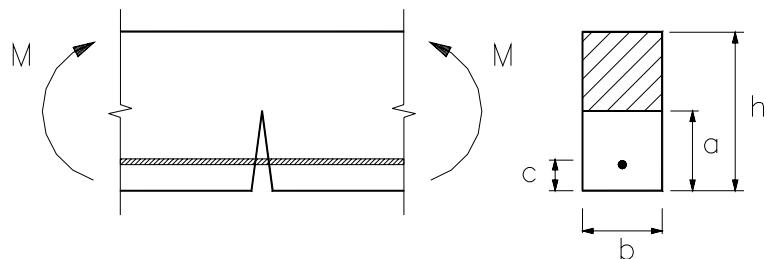
# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS



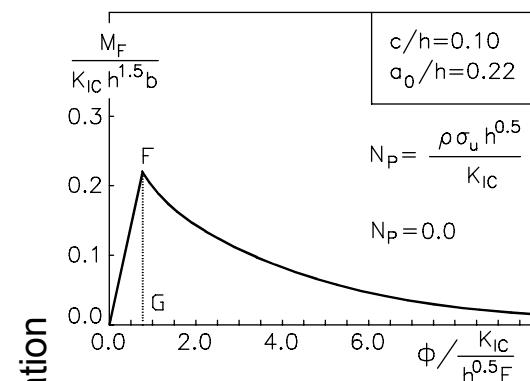
$$N_p = \frac{\rho \sigma_u h^{0.5}}{K_{IC}}$$

(Carpinteri 1981, 1984; Massabò, 1994)

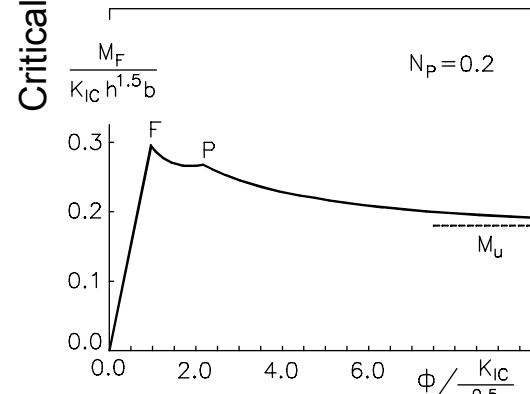
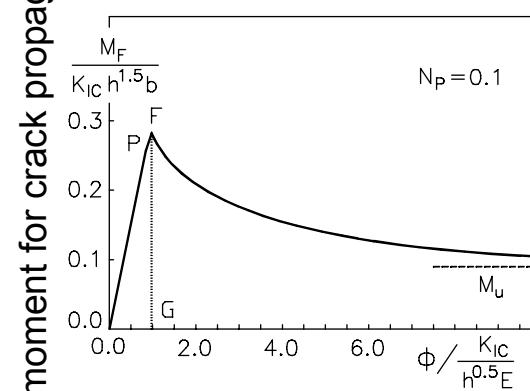
# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS



(Carpinteri 1981, 1984; Massabò, 1994)

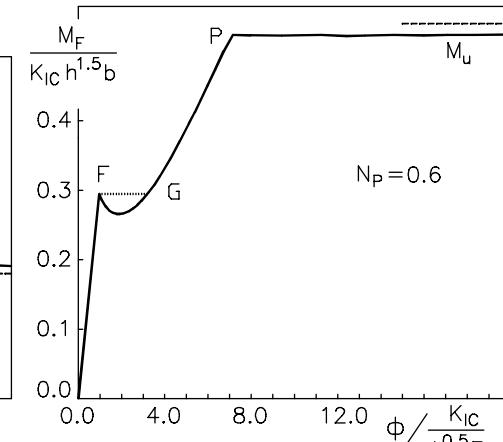
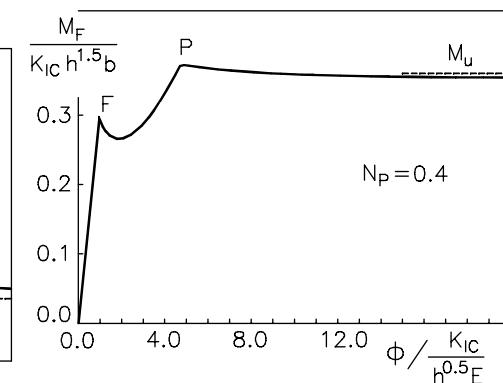


Critical moment for crack propagation

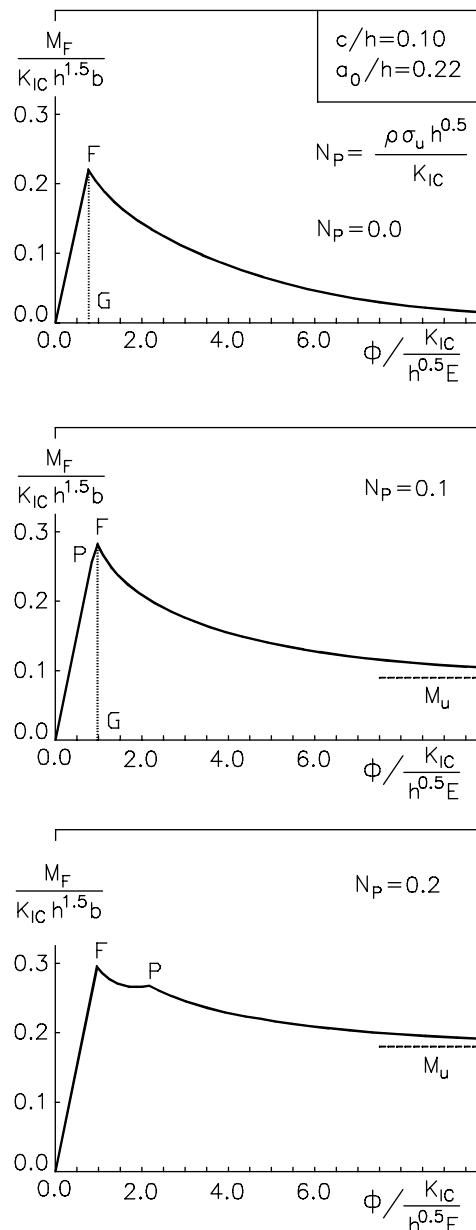
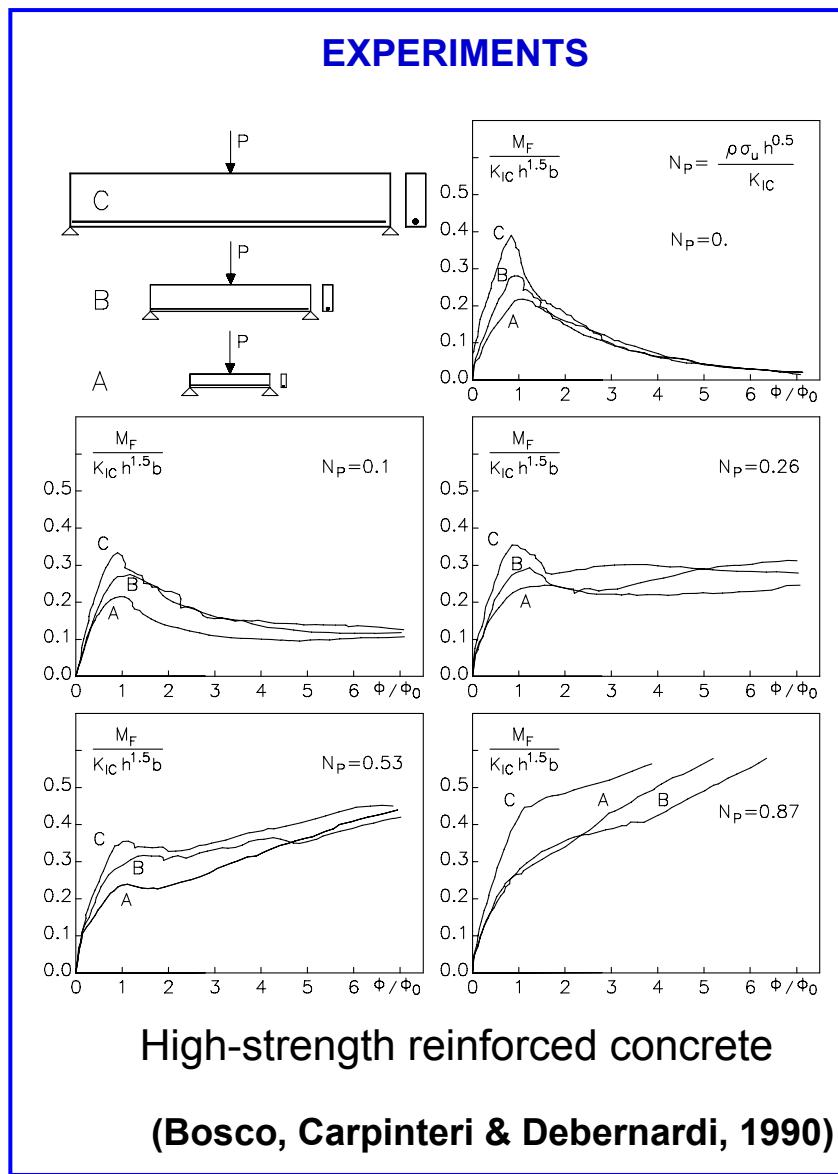


Localized rotation

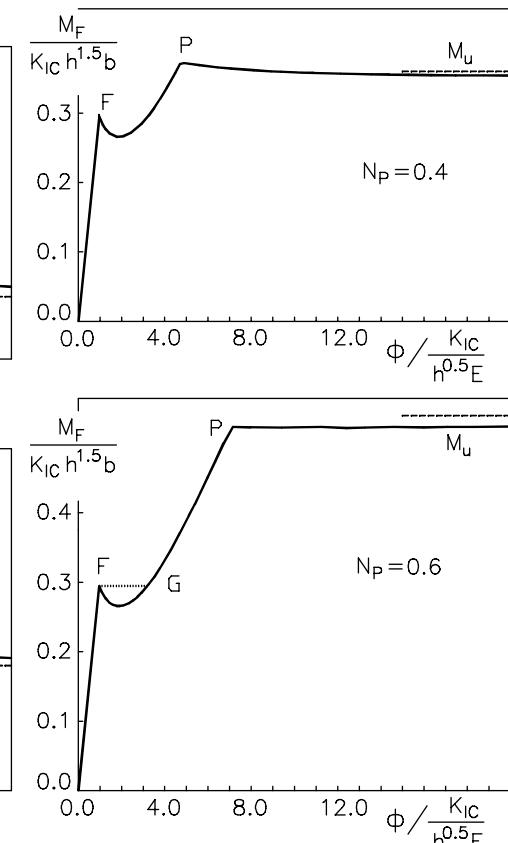
$$N_P = \frac{\rho \sigma_u h^{0.5}}{K_{IC}}$$



# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS



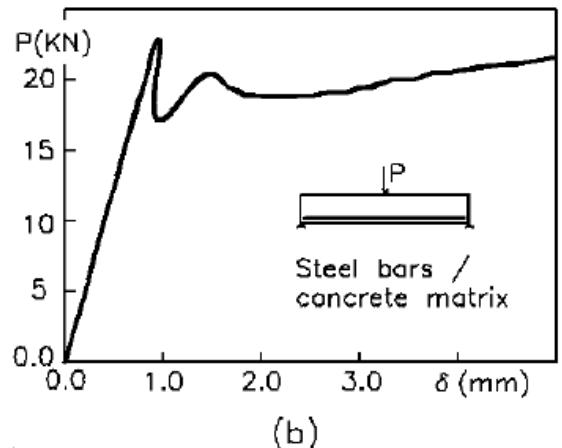
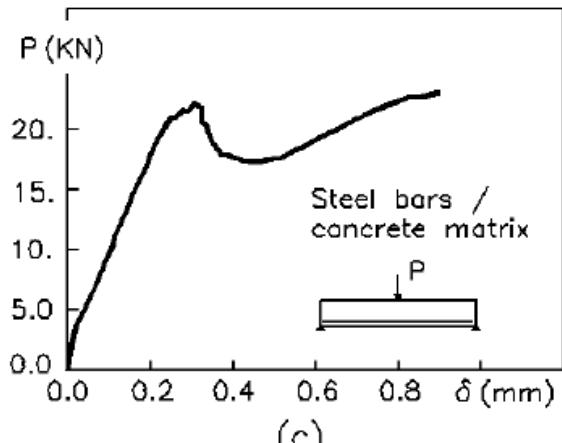
$$N_p = \frac{\rho \sigma_u h^{0.5}}{K_{IC}}$$



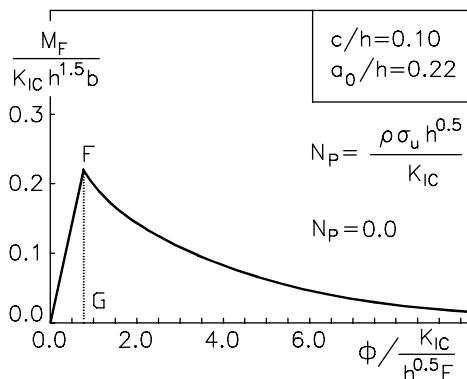
Localized rotation

# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS

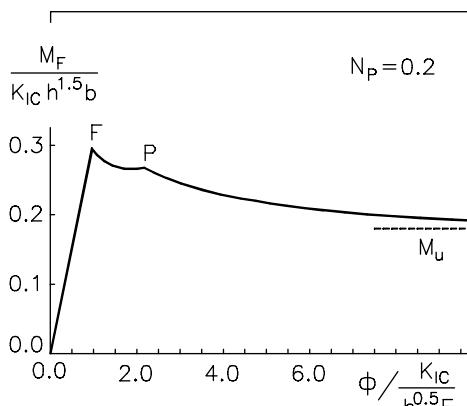
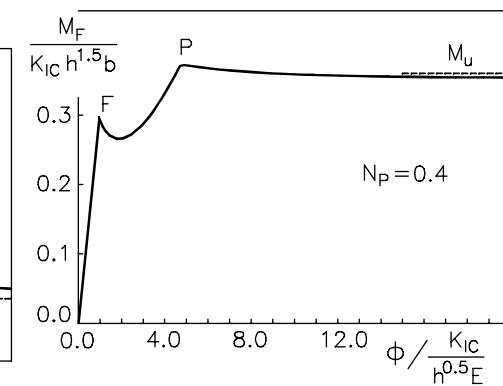
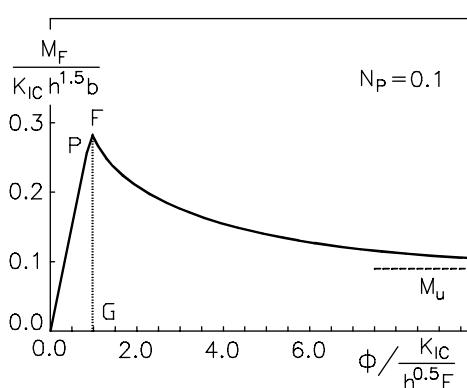
## EXPERIMENTS



(Levi, Bosco & Debernardi, 1998  
Bosco, Carpinteri & Debernardi, 1990)



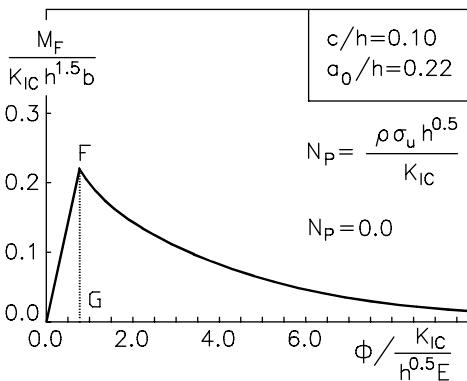
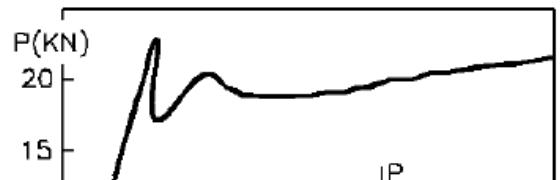
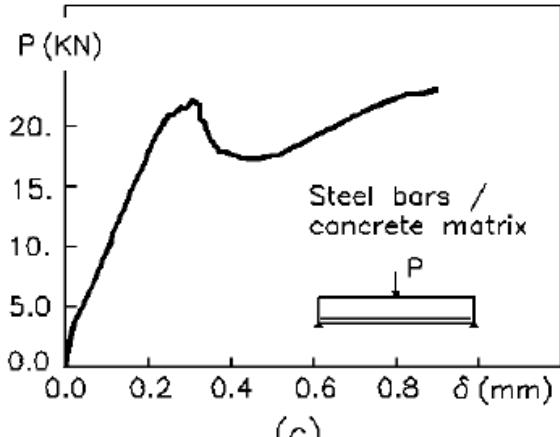
$$N_P = \frac{\rho \sigma_u h^{0.5}}{K_{IC}}$$



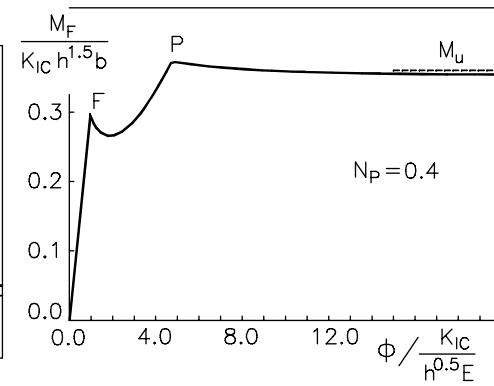
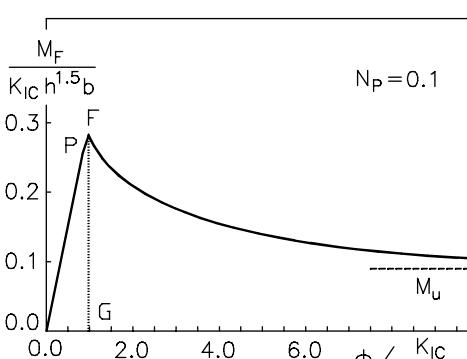
Localized rotation

# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS

## EXPERIMENTS



$$N_P = \frac{\rho \sigma_u h^{0.5}}{K_{IC}}$$

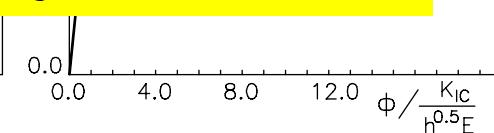
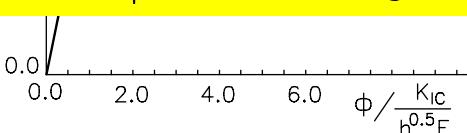


Flexural response of brittle matrix composites with ductile reinforcements:

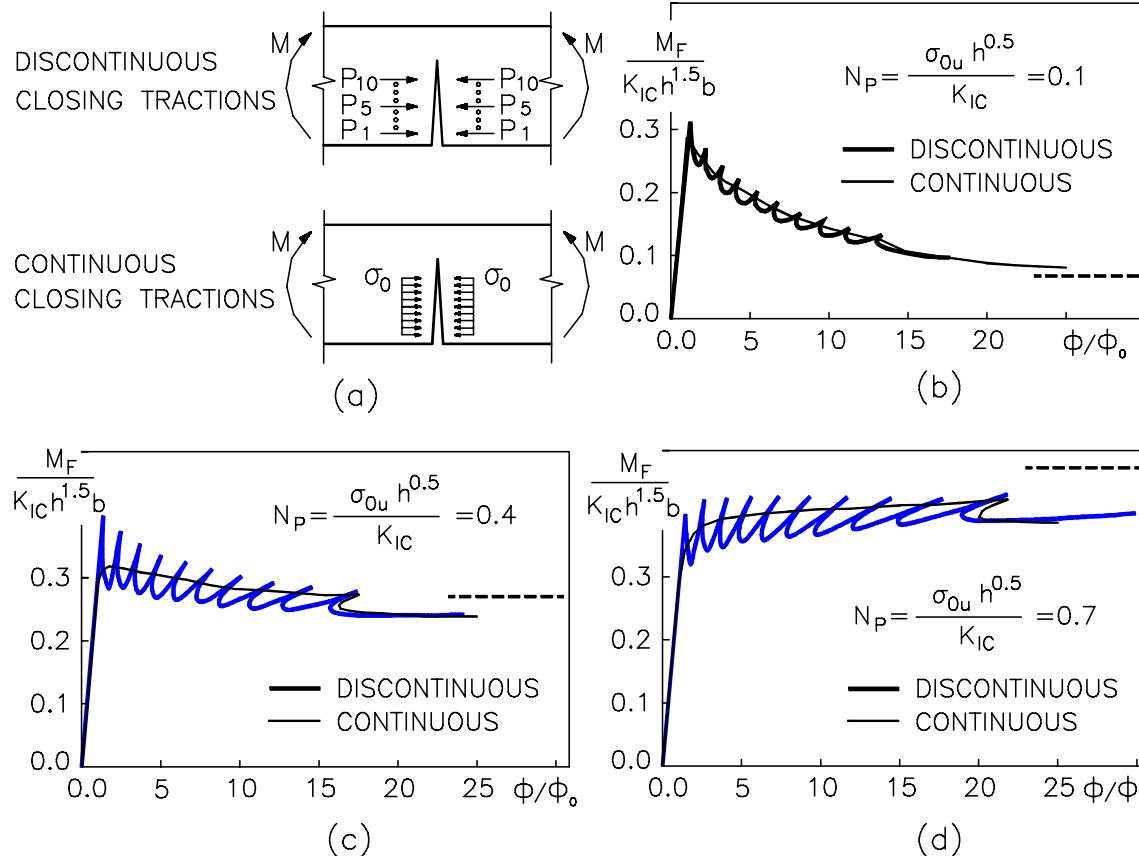
- controlled by a single brittleness number
- brittle to ductile transition on increasing the beam depth
- local snap-through instabilities can be explained using bridged-crack model

(L)

Bosco, Carpinteri & Debernardi, 1990)

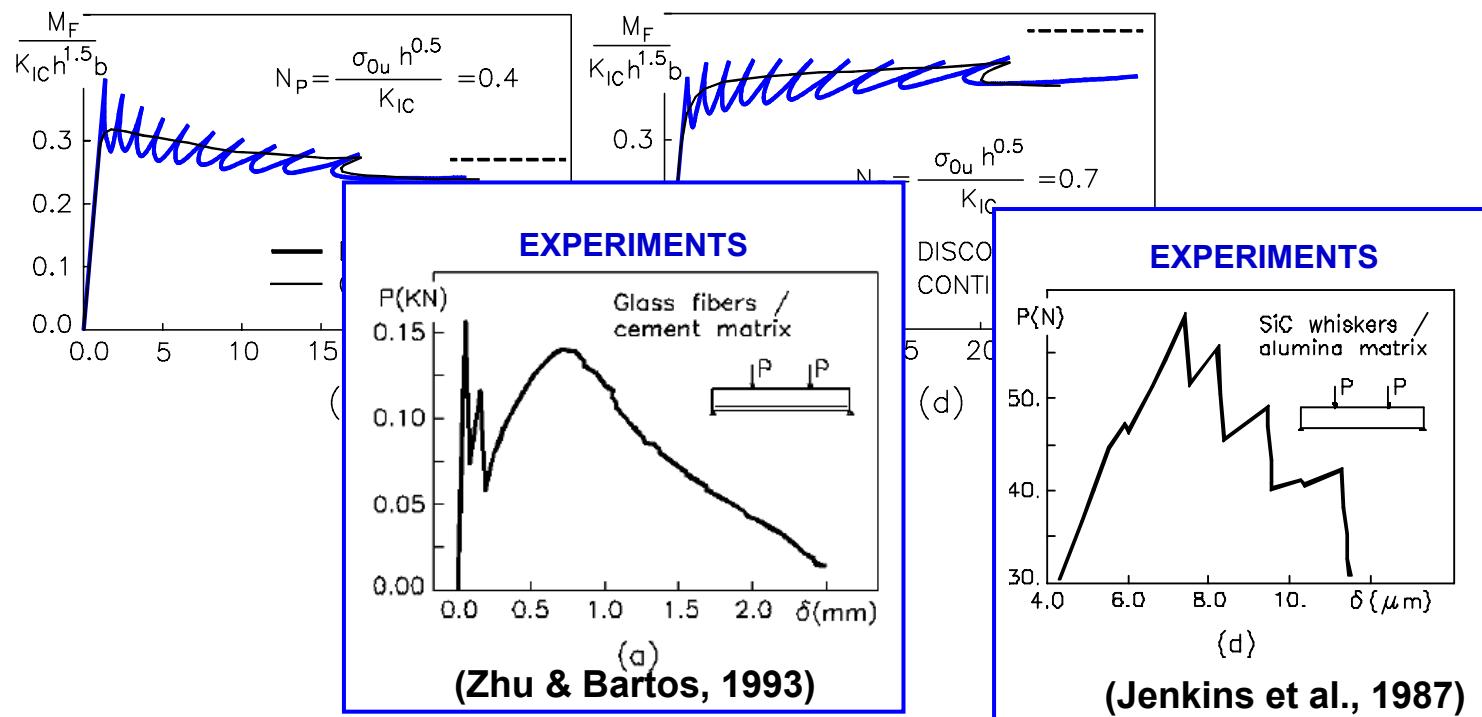
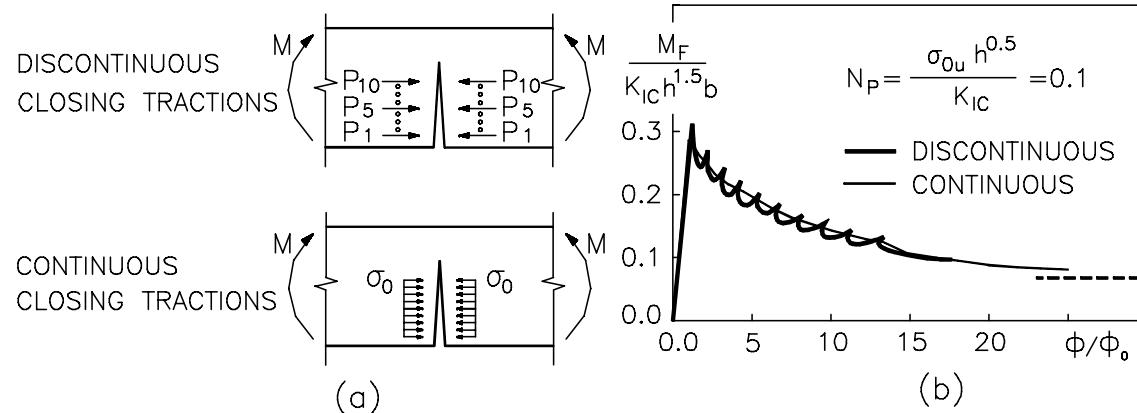


# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS

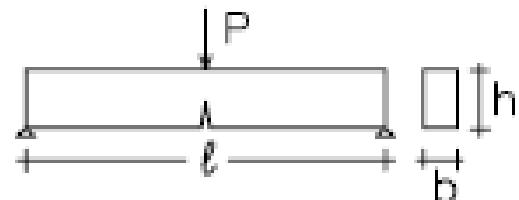


(Carpinteri & Massabò, 1997)

# FLEXURAL RESPONSE OF BRITTLE MATRIX COMPOSITES WITH DISCRETE DUCTILE REINFORCEMENTS



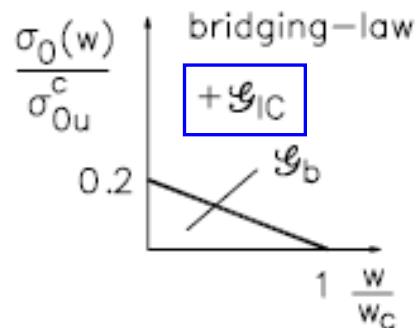
## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES



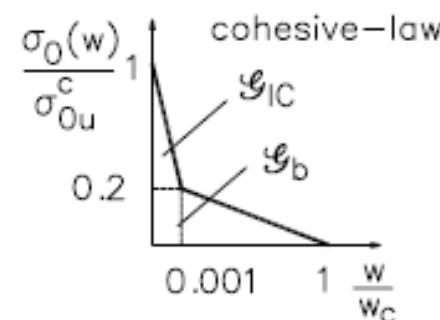
$$l/h = 6 \quad a_0/h = 0.1$$

Fiber reinforced brittle matrix composite  
(e.g. fiber reinforced high strength concrete;  
fiber reinforced ceramic)

Bridged-crack model



Cohesive-crack model

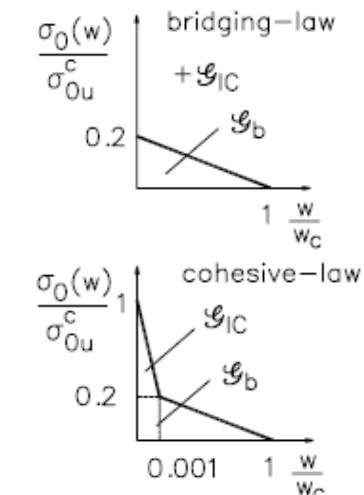
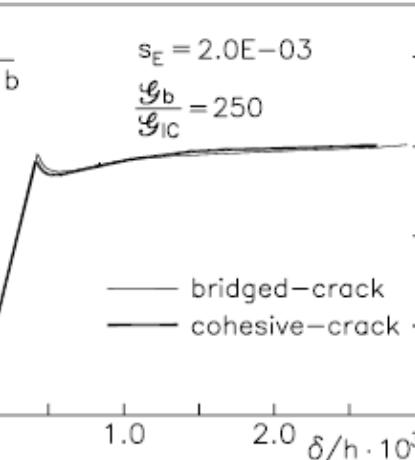
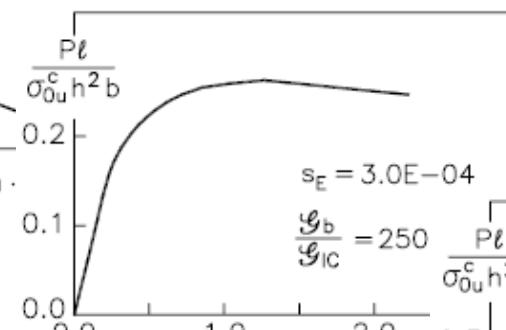
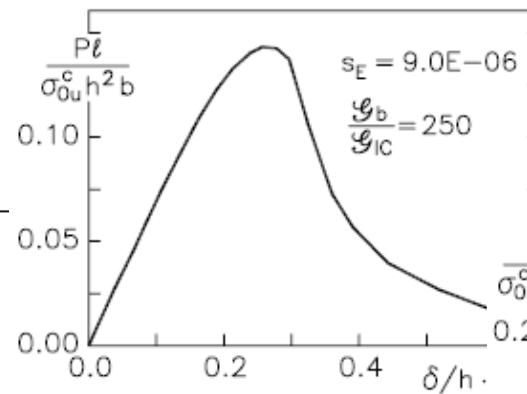
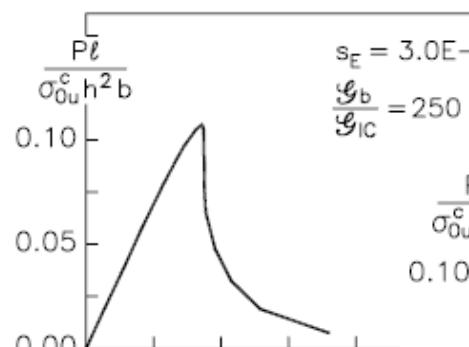


Two dimensionless groups govern mechanical behavior

$$S_E = \frac{G^c}{\sigma_{0u}^c h} = \frac{G_b + G_{IC}}{\sigma_{0u}^c h} \quad \frac{G_b}{G_{IC}} = 250$$

(brittleness number, Carpinteri 1985)

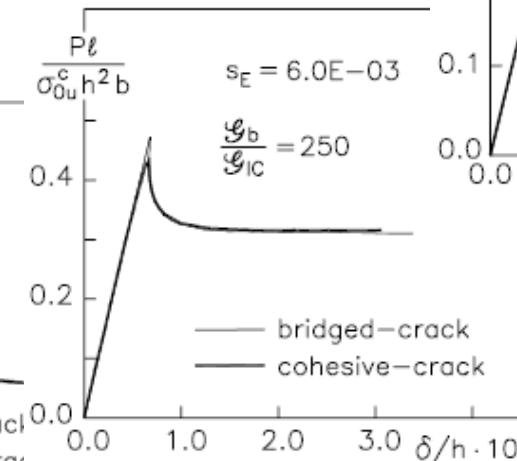
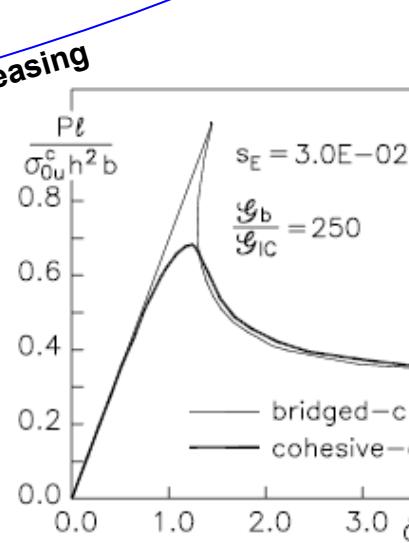
## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES



$$s_E = \frac{G^c}{\sigma_{0u}^c h} = \frac{(G_b + G_{IC})}{\sigma_{0u}^c h}$$

$$\frac{G_b}{G_{IC}}$$

$s_E$  increasing



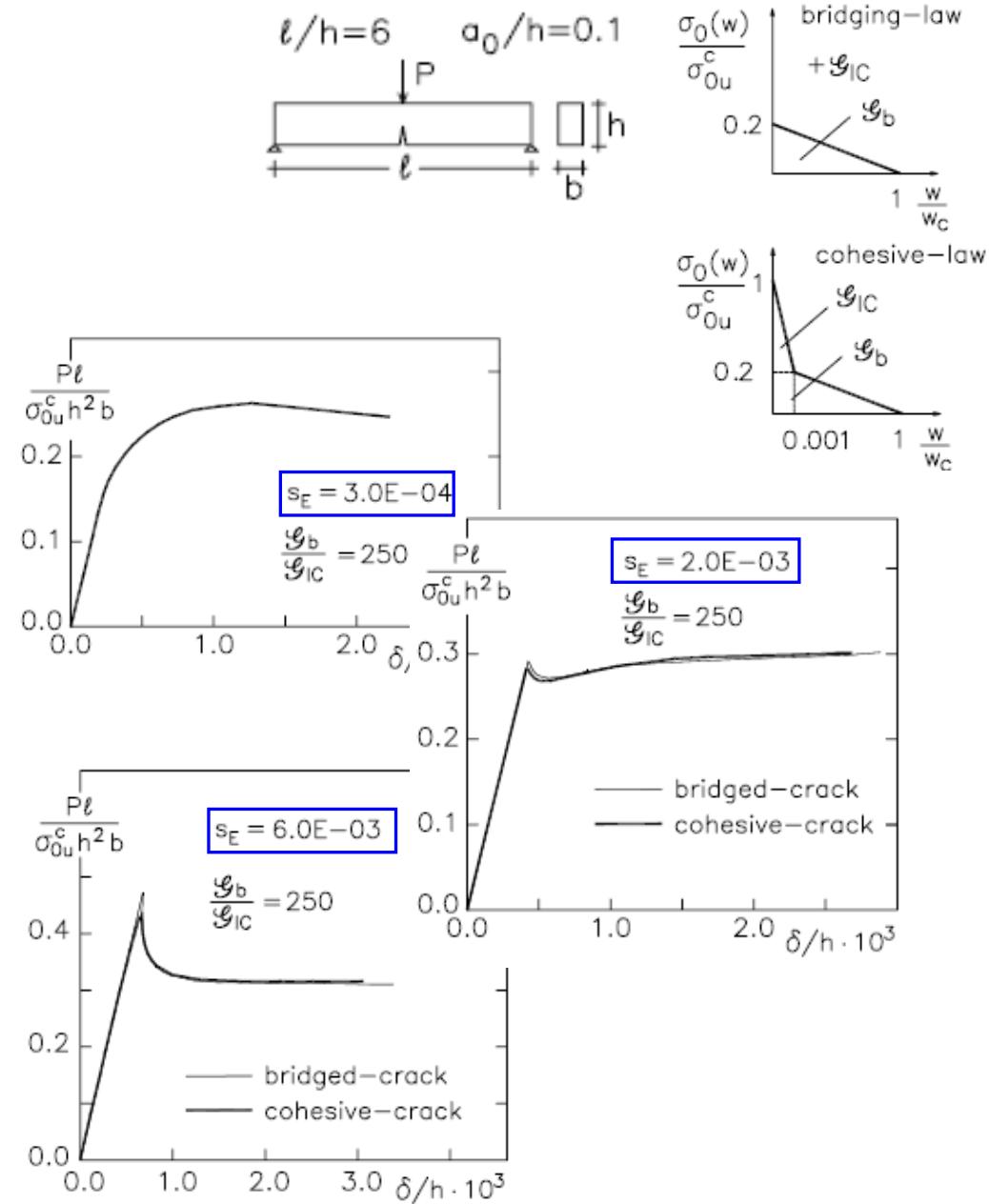
(Massabo, 1999;  
Carpinteri & Massabò, 1997)

## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES

$$s_E = \frac{G^c}{\sigma_{0u}^c h} = \frac{G_b + G_{IC}}{\sigma_{0u}^c h}$$

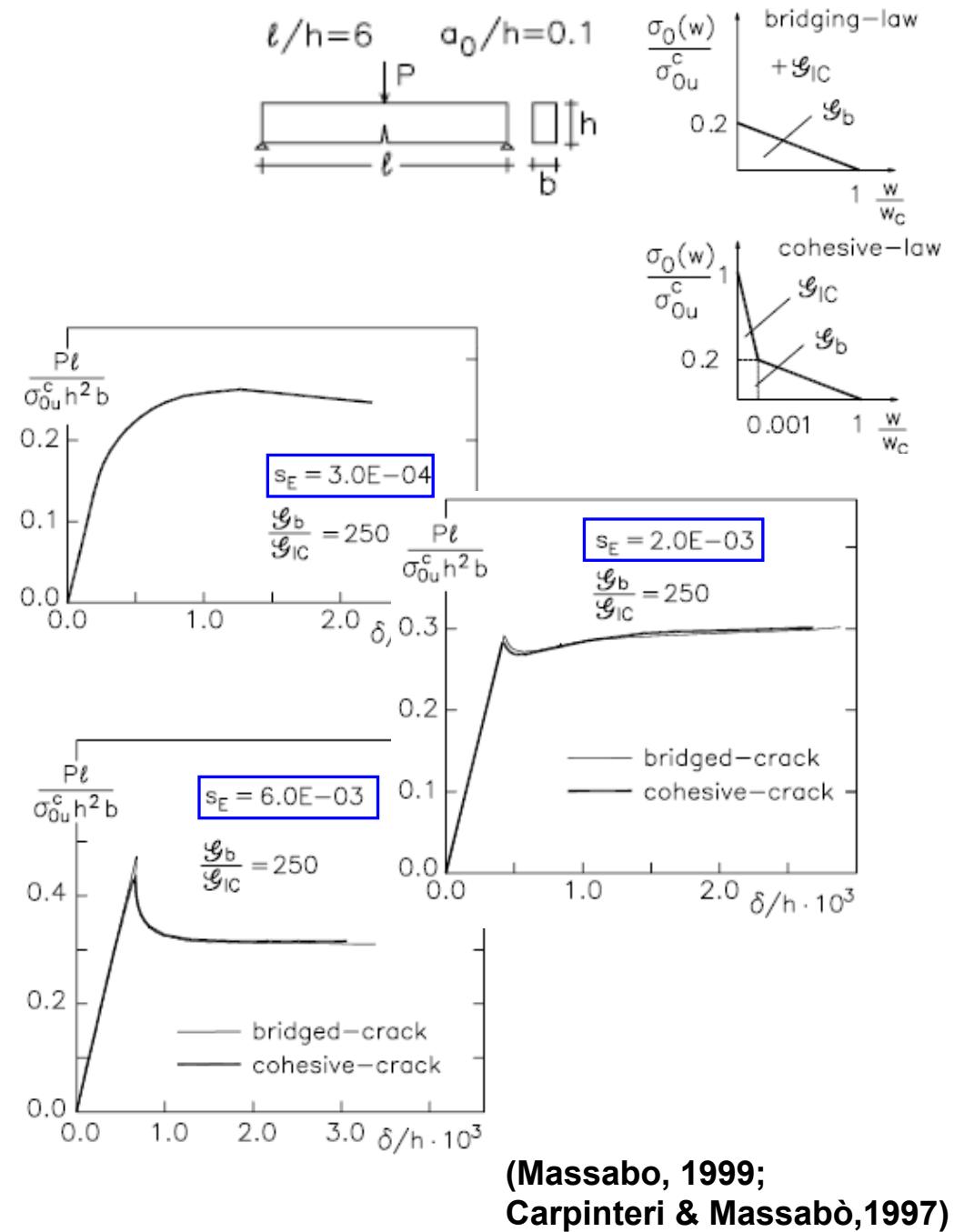
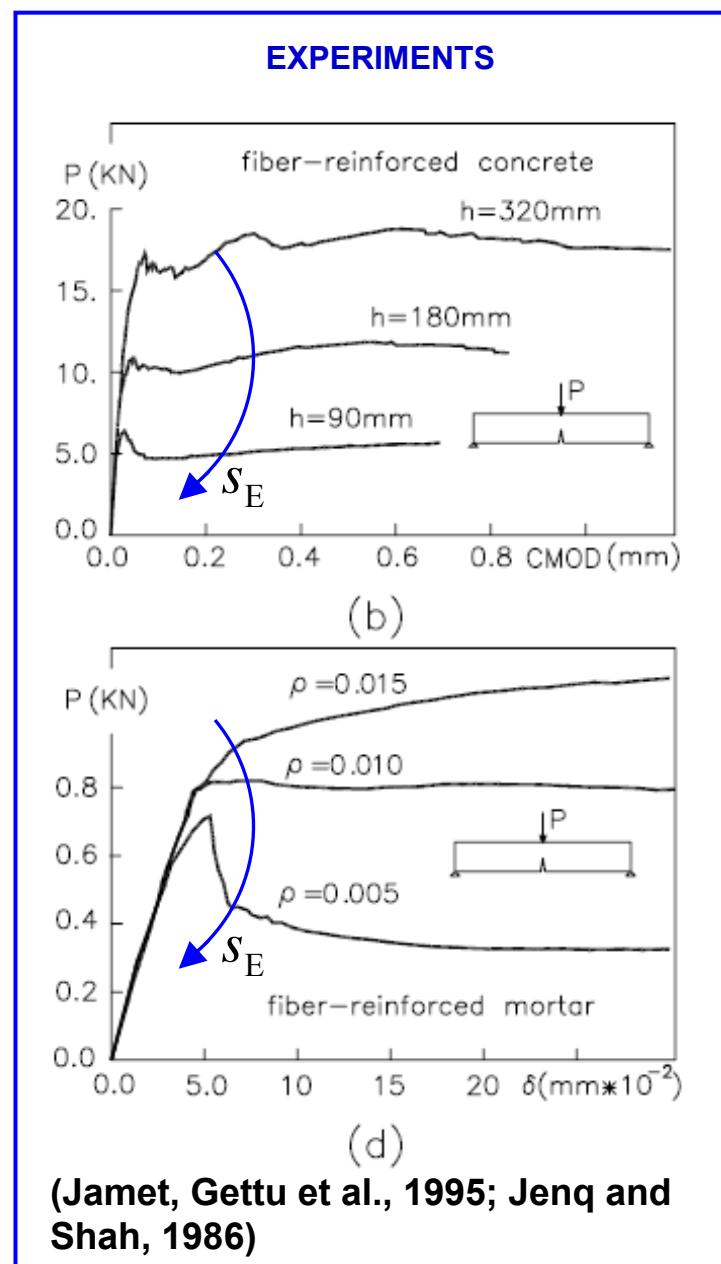
$\frac{G_b}{G_{IC}} = 250$

$s_E$  increasing

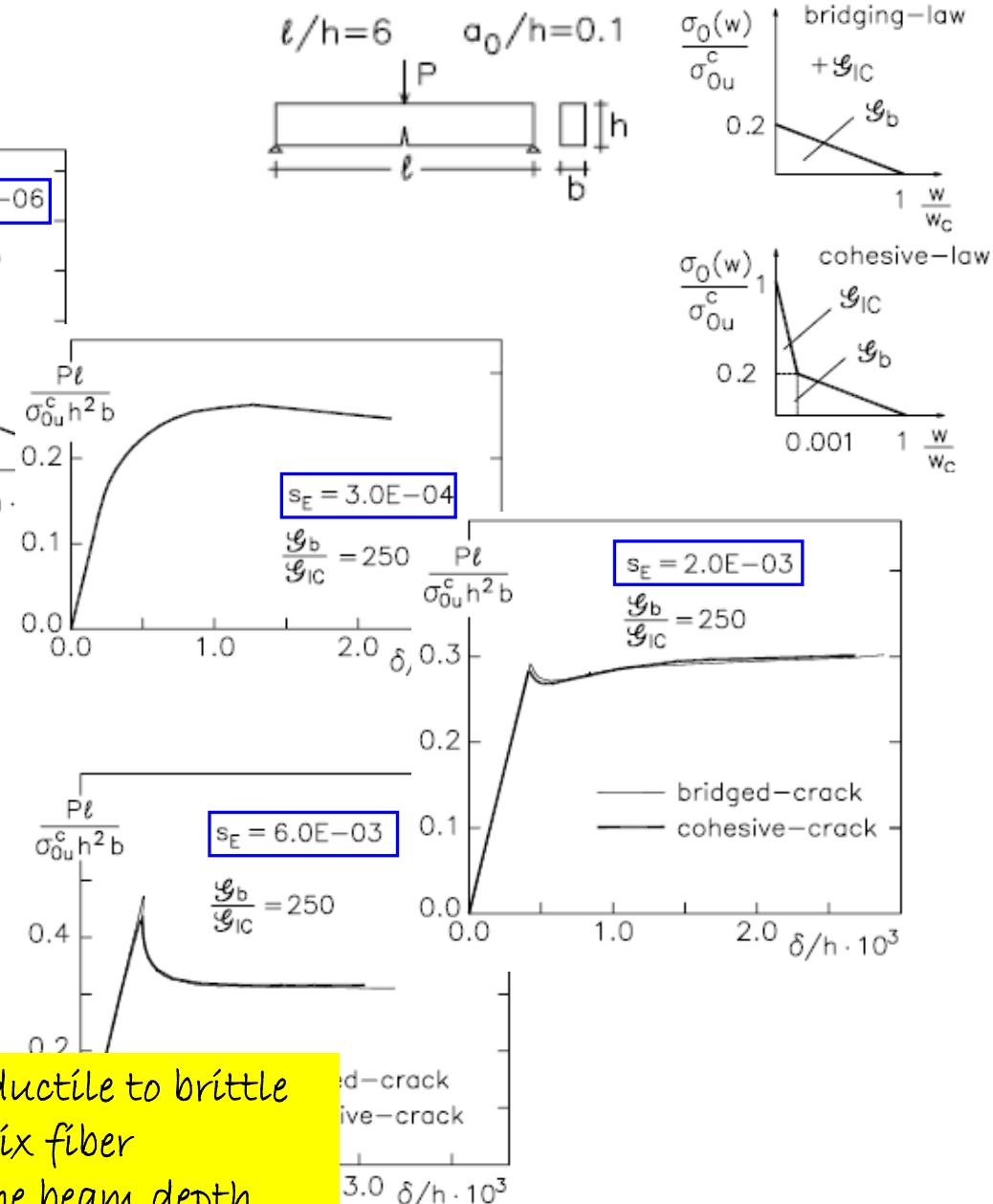
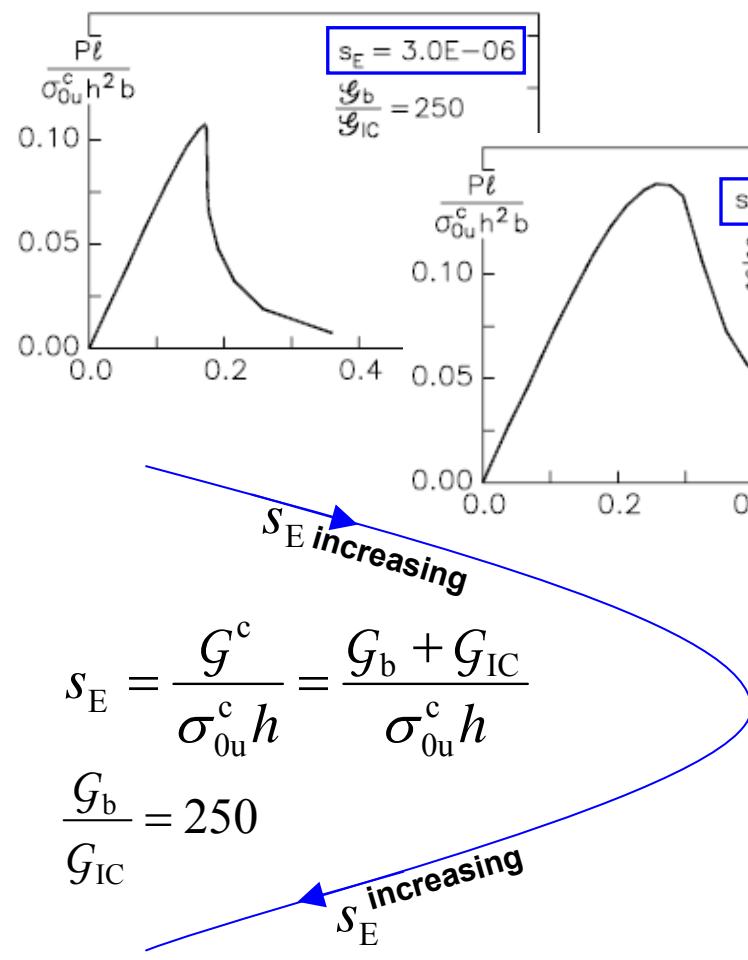


(Massabo, 1999;  
Carpinteri & Massabò, 1997)

# TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES

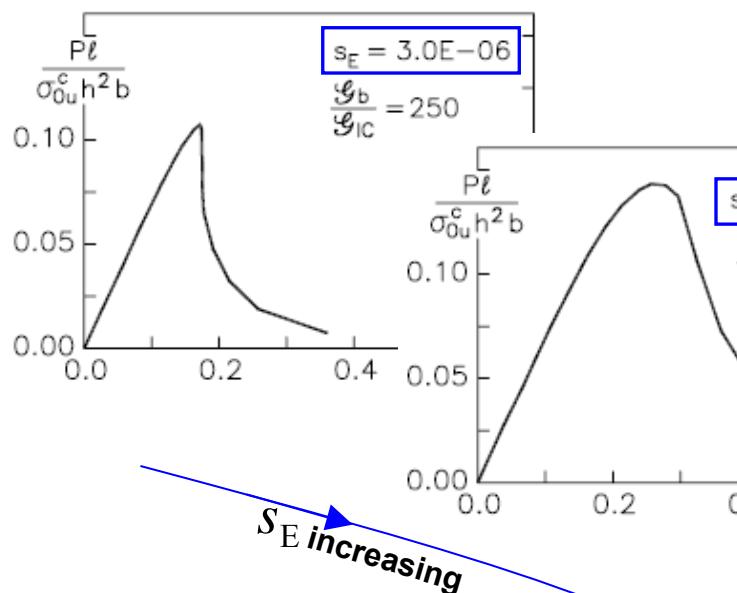


## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES



(Massabo, 1999;  
Carpinteri & Massabò, 1997)

## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES

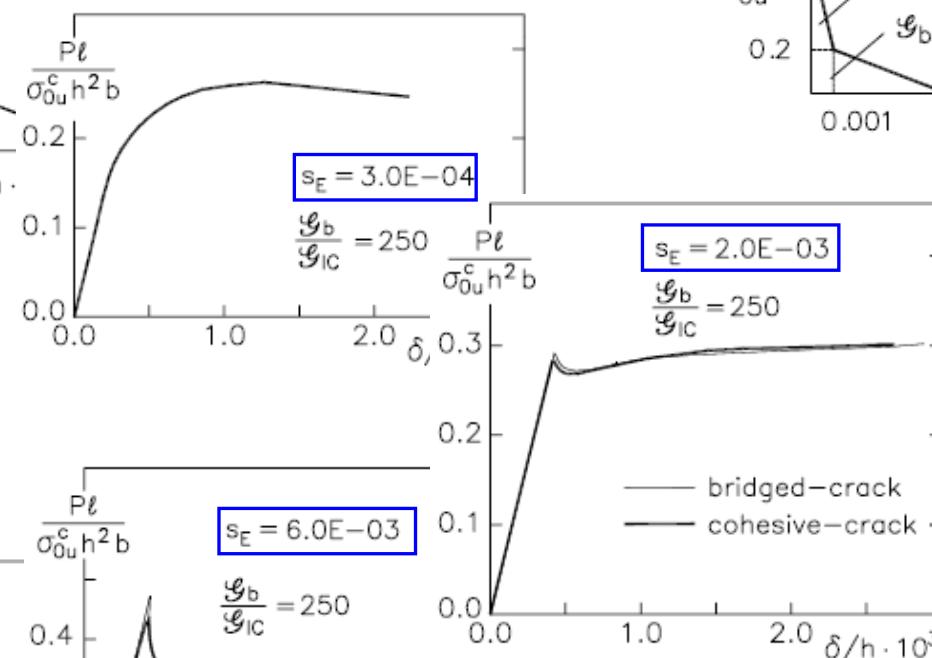
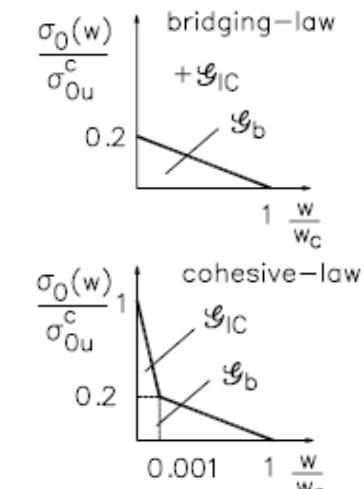
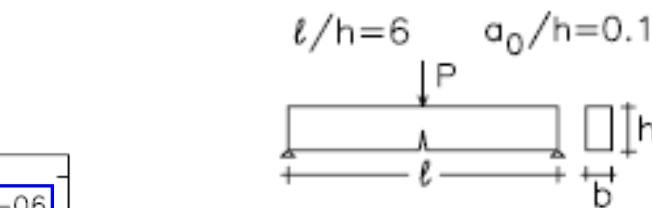
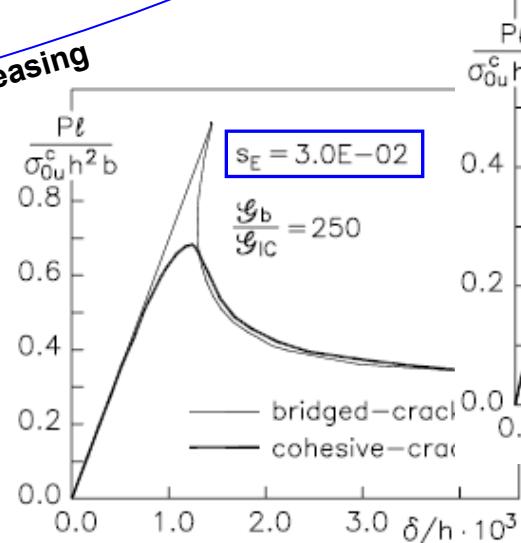


$s_E$  increasing

$$s_E = \frac{G^c}{\sigma_{0u}^c h} = \frac{G_b + G_{IC}}{\sigma_{0u}^c h}$$

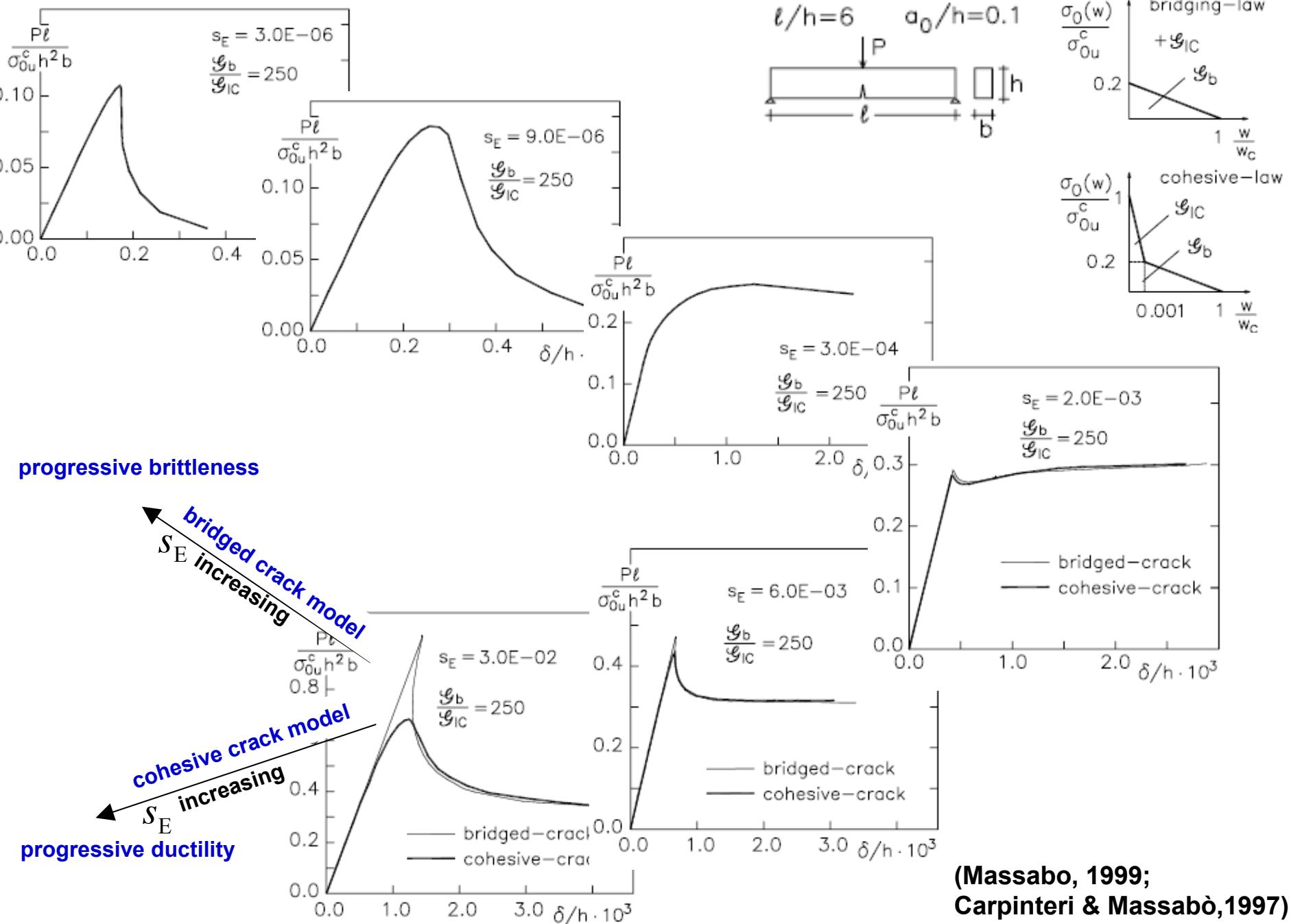
$$\frac{G_b}{G_{IC}} = 250$$

$s_E$  increasing



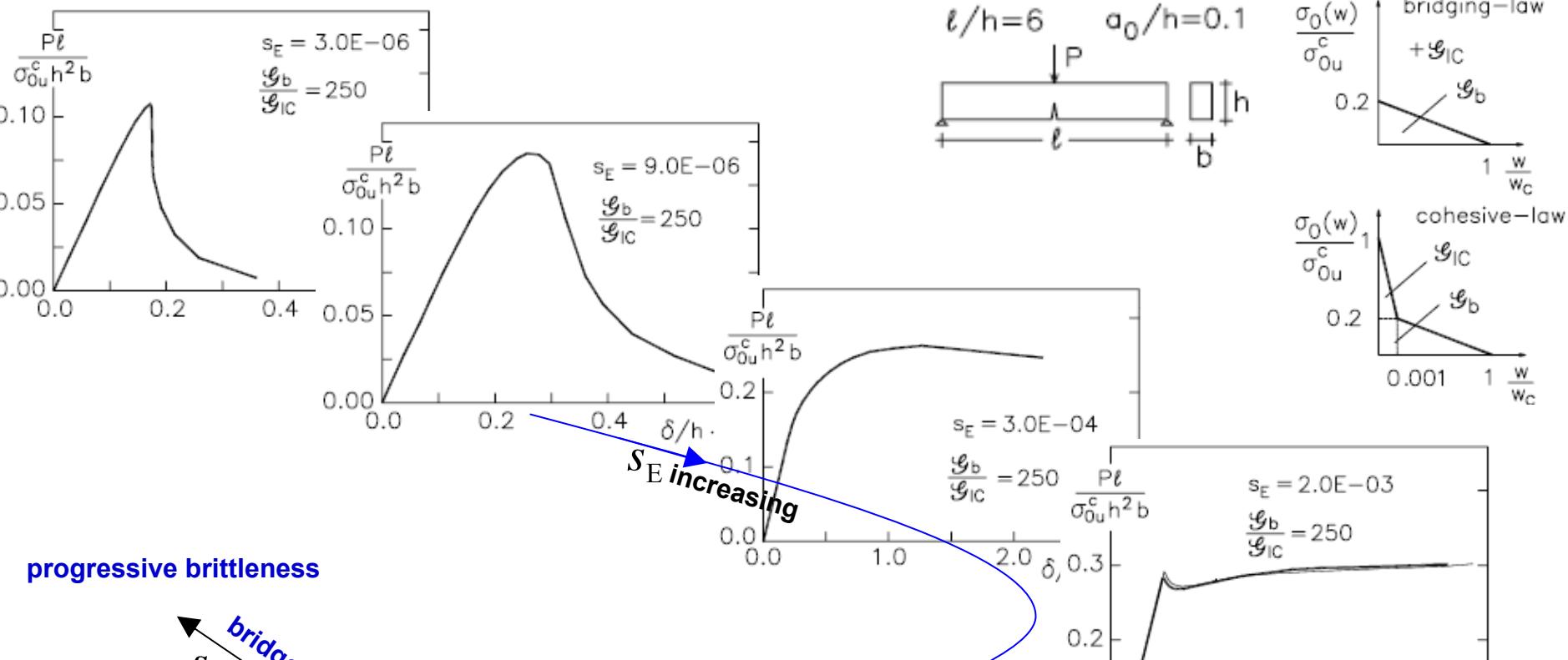
(Massabo, 1999;  
Carpinteri & Massabò, 1997)

# TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES



(Massabo, 1999;  
Carpinteri & Massabò, 1997)

## TRANSITIONS IN THE FLEXURAL BEHAVIOR OF FIBER REINFORCED COMPOSITES

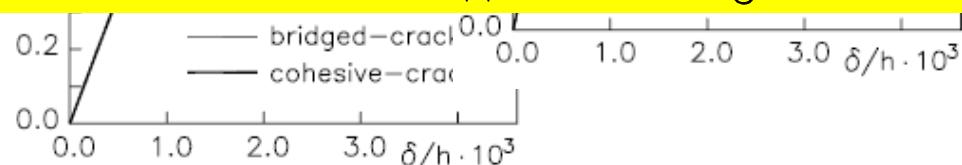


progressive brittleness

Drawbacks:  
bridged-crack

- The cohesive crack model is computationally very expensive
- The bridged crack model has limited range of applicability when matrices are not perfectly brittle
- Cohesive crack model must be applied to study crack initiation

progressive ductility



(Massabo, 1999;  
Carpinteri & Massabò, 1997)

Università degli Studi di Pisa  
15 Febbraio 2010

## **PROBLEMI DI FRATTURA E DANNEGGIAMENTO IN MATERIALI E SISTEMI COMPOSITI**

**Parte 2:**

### **INTERAZIONE DI MECCANISMI MULTIPLI DI DANNEGGIAMENTO**

**Roberta Massabò**

Università degli Studi di Genova



**unige**  
UNIVERSITÀ  
DEGLI STUDI  
DI GENOVA

DICAT - Department of Civil, Environmental and Architectural Engineering

## **PEOPLE:**

**B.N. Cox** (Teledyne Scientific, CA, U.S.A.)

**A. Cavicchi** (post doc, University of Genova, Italy)

**M.G. Andrews** (former grad. student, Northwestern University, Boeing Co. U.S.A)

**F. Campi** (grad. student, University of Genova, Italy)

## **FUNDING:**

**U.S. Office of Naval Research, N00014-05-1-0098**

**U.S. Army Research Office, DAAD19-99-C-0042**

**Italian MIUR**

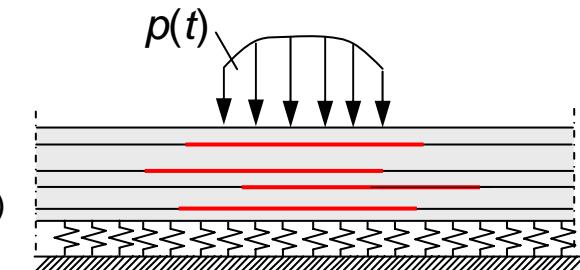
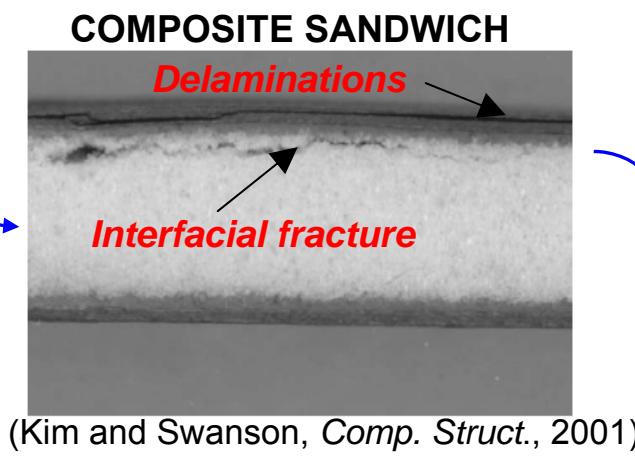
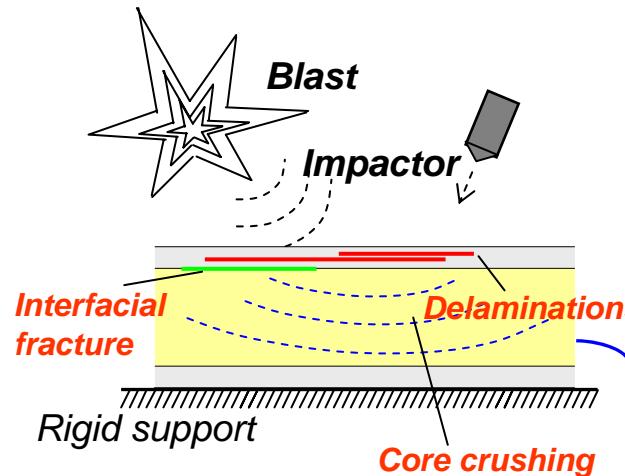
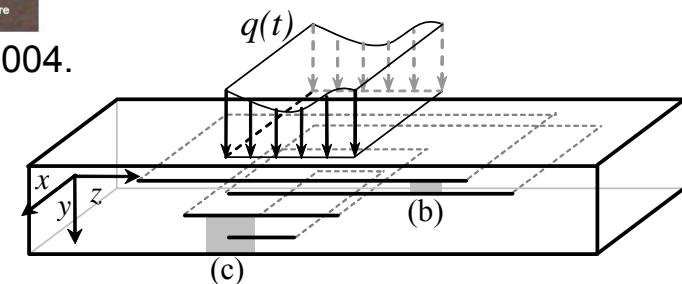
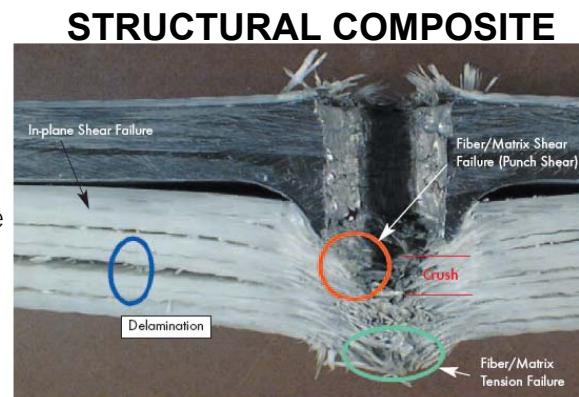
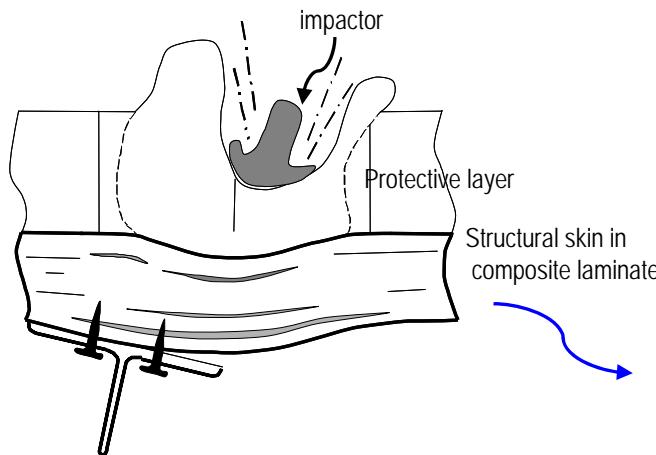
## **OUTLINE**

**-Introduction and motivation**

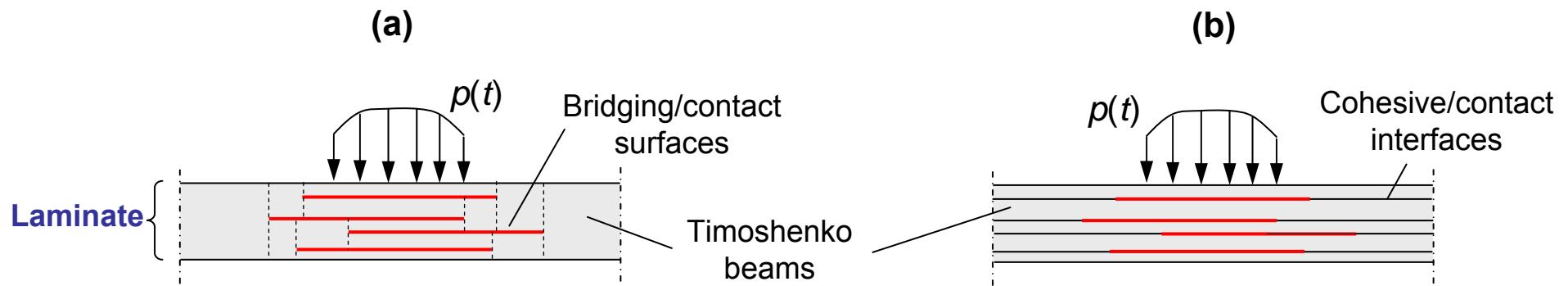
**- Theoretical models to study the interaction of multiple damage mechanisms in laminates and composite sandwiches;**

**- Relevant results**

# MULTIPLE DAMAGE MECHANISMS IN LAMINATES AND SANWICHES



# MECHANICAL MODELS COMPOSITE LAMINATES AND MULTILAYERED SYSTEMS



**Convenient for  
homogeneous beams and  
static loading**

(Andrews, Massabò & Cox, *IJSS*, 2006)

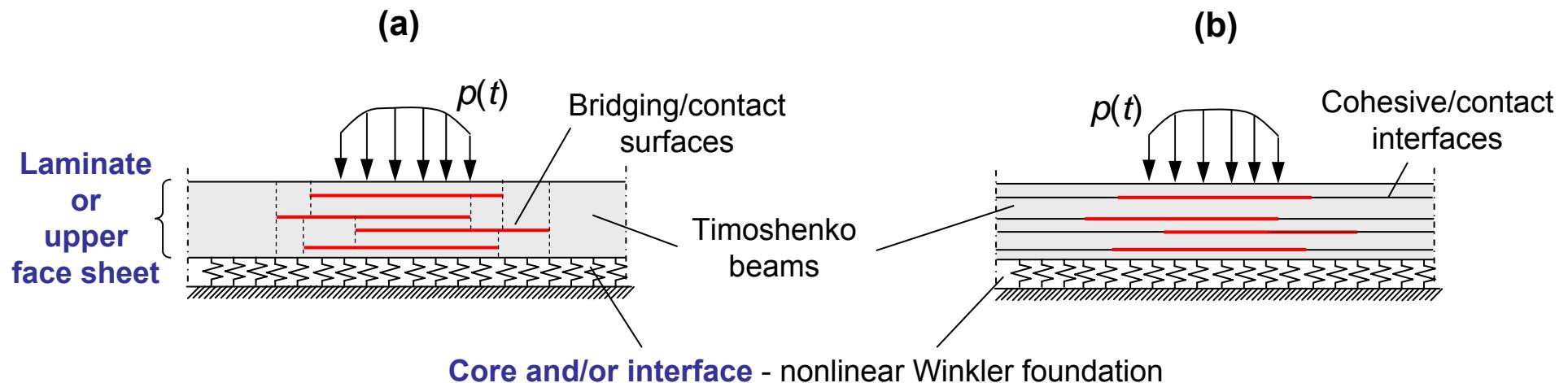
(Andrews & Massabò, *EFM*, 2007)

**Convenient for  
multilayered beams and  
dynamic loading**

(Andrews, Massabò, Cavicchi & Cox, *IJSS*, 2009)

(Andrews & Massabò, *Comp. A*, 2008)

# MECHANICAL MODELS FULLY BACKED COMPOSITE SANDWICHES



**Convenient for  
homogeneous beams and  
static loading**

(Andrews, Massabò & Cox, *IJSS*, 2006)

(Andrews & Massabò, *EFM*, 2007)

(Cavicchi and Massabò, 2009, proc. ICCM17)

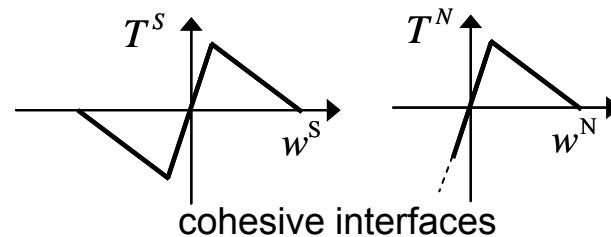
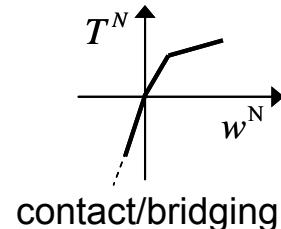
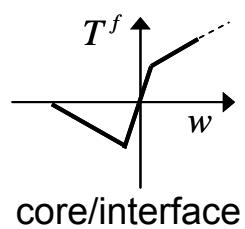
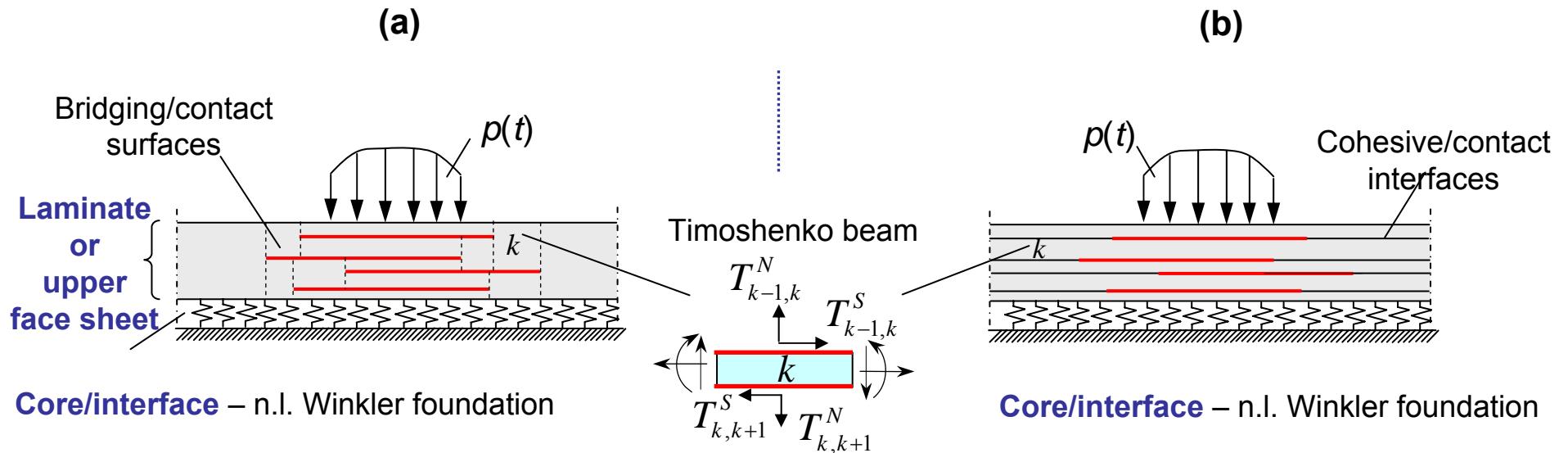
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(Andrews & Massabò, *Comp. A*, 2008)

(Cavicchi and Massabò, 2009, proc. AIMETA)

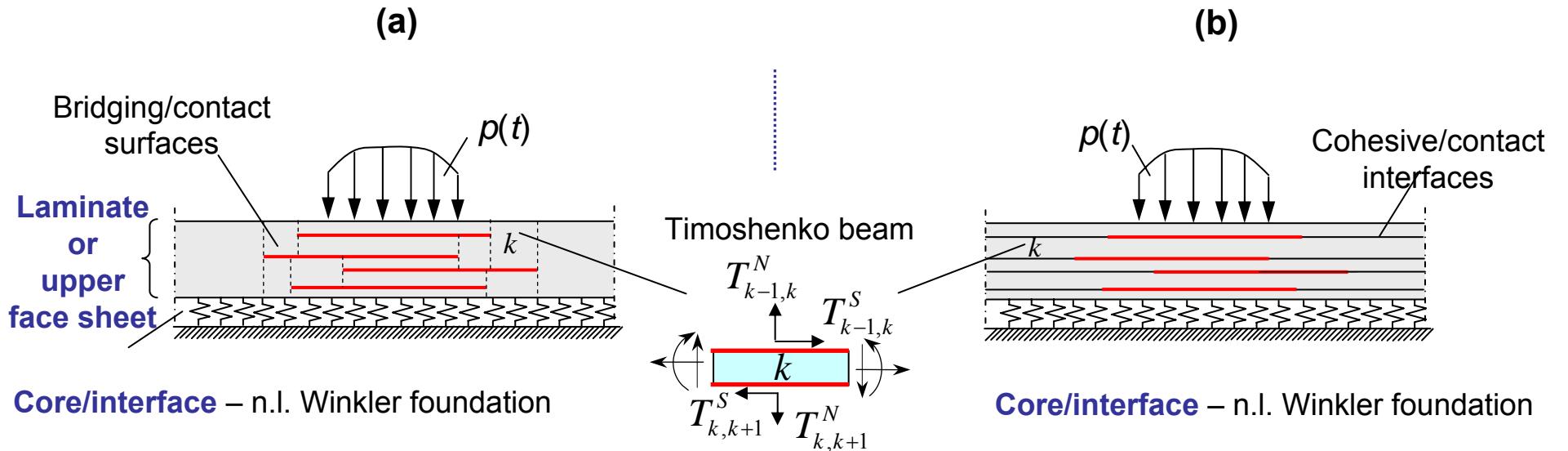
# MECHANICAL MODELS



- Homogeneous, orthotropic, linear elastic skin
- Perfectly brittle matrix + crack bridging/contact
- Accurate mode decomposition (Andrews & Massabò, *EFM*, 2007)

- Multi-layered, orthotropic, linear elastic skin
- Cohesive interfaces: perfect adhesion, brittle or cohesive fracture, contact, bridging

# MECHANICAL MODELS



Generalized displacements

$$u_k, w_k, \varphi_k$$

Dynamic equilibrium

$$N'_k - T_{k,k+1}^S + T_{k-1,k}^S = \rho_m S_k \ddot{u}_k$$

$$M'_k - V_k - 1/2 h_k (T_{k,k+1}^S + T_{k-1,k}^S) = \rho_m I_k \ddot{\varphi}_k$$

$$V'_k + T_{k,k+1}^N - T_{k-1,k}^N = \rho_m S_k \ddot{w}_k$$

Compatibility

$$\varepsilon_k = u'_k$$

$$\varphi_k = -w'_k + \gamma_k$$

$$\chi_k = \varphi'_k$$

Constitutive laws

$$N_k = A_k \varepsilon_k$$

$$M_k = D_k \chi_k$$

$$V_k = G_k \gamma_k$$

## SOLUTION METHOD

semi-analytic (iterative procedure needed to define contact/bridging regions and plastically admissible fields)

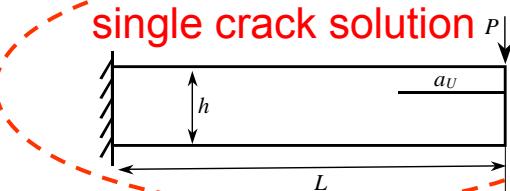
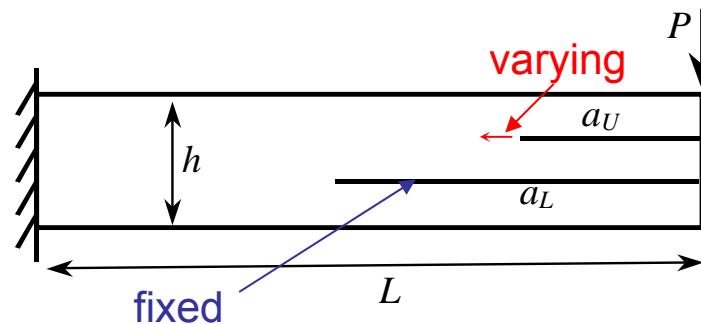
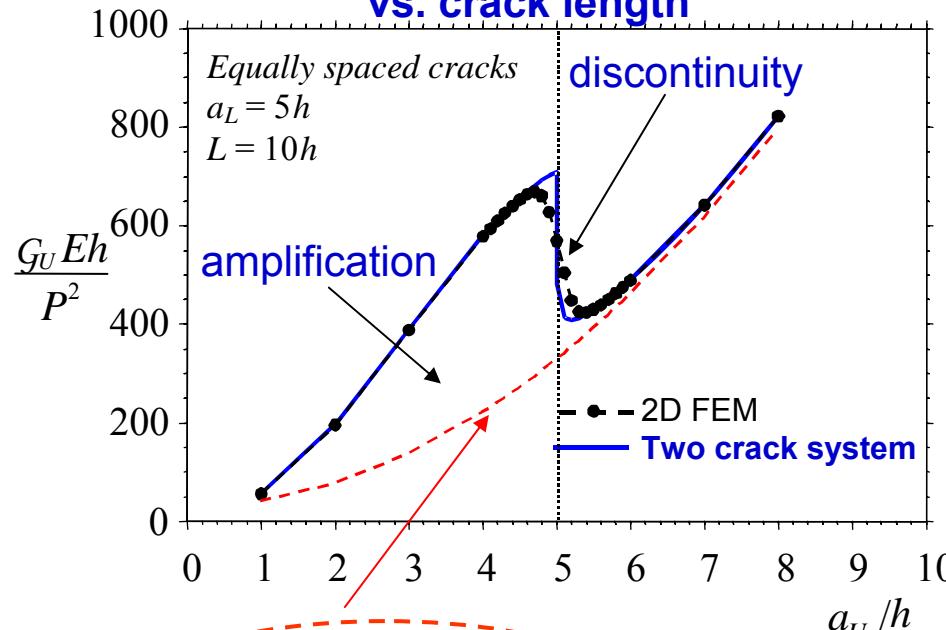
## SOLUTION METHOD

numerical: time/space discretization, finite difference solution scheme

# INTERACTION EFFECTS – QUASI STATIC LOADING AMPLIFICATION AND SHIELDING OF FRACTURE PARAMETERS

(Andrews, Massabò and Cox, IJSS, 2006; Andrews and Massabò, Comp. A, 2008)

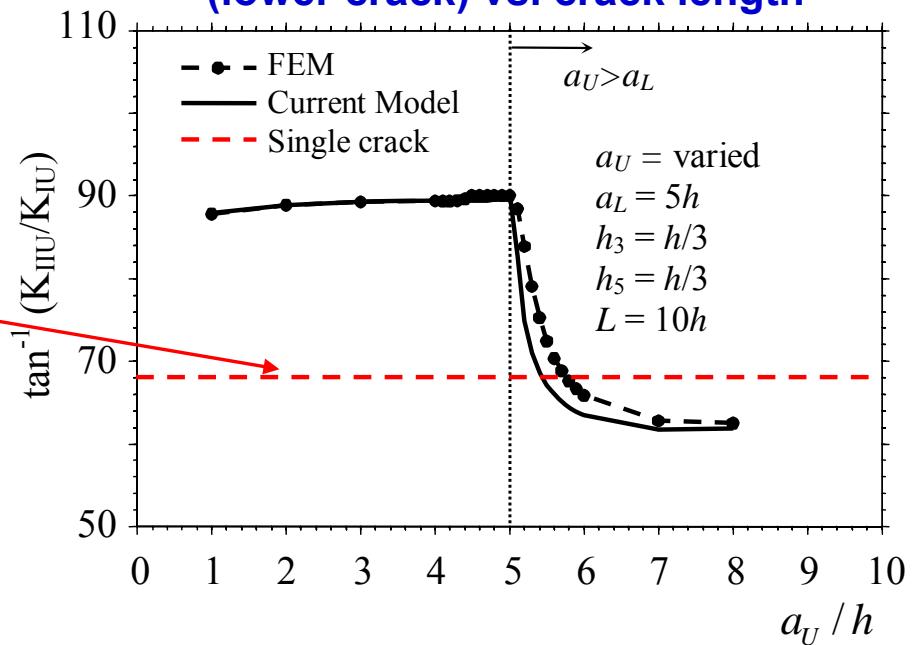
**Energy release rate of upper crack  
vs. crack length**



**Assumptions:**

- System of two cracks in a cantilever beam
- Frictionless contact
- Homogeneous, isotropic and perfectly brittle material

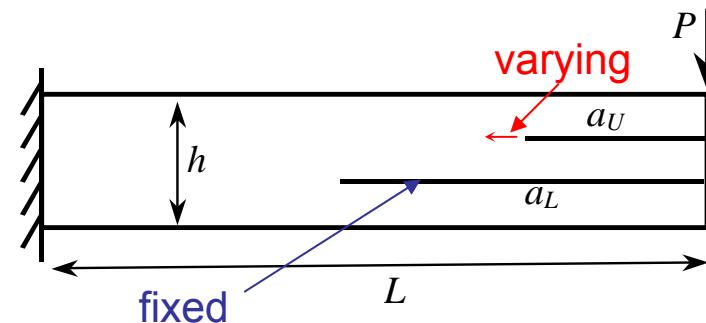
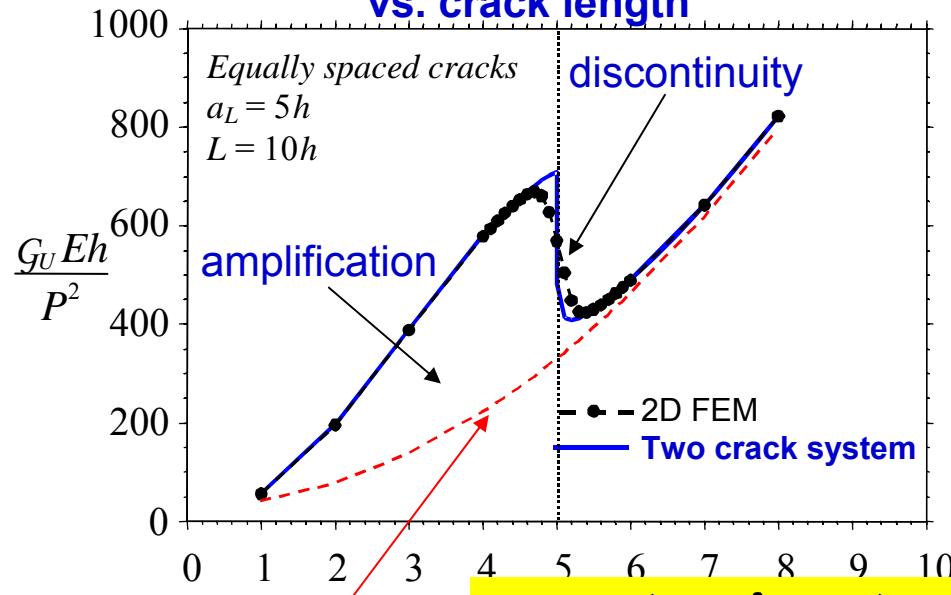
**Relative amount of mode II to mode I  
(lower crack) vs. crack length**



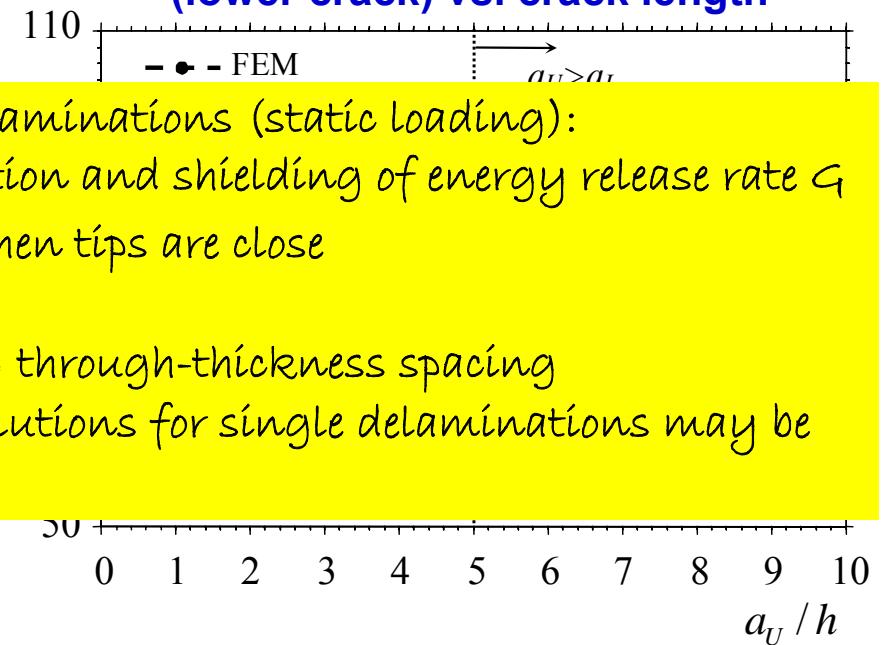
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**Energy release rate of upper crack  
vs. crack length**



**Relative amount of mode II to mode I  
(lower crack) vs. crack length**



Interaction of multiple delaminations (static loading):

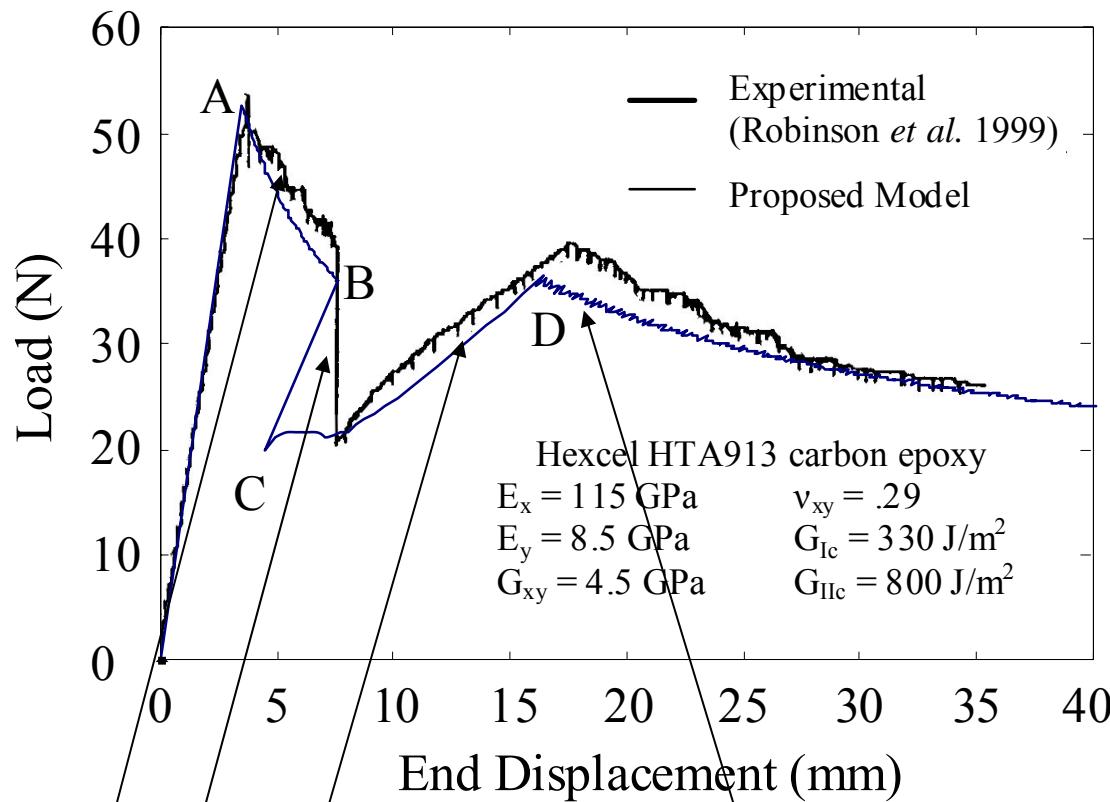
- phenomena of amplification and shielding of energy release rate  $G$
- sudden changes in  $G$  when tips are close
- mode ratio variations
- phenomena controlled by through-thickness spacing
- predictions based on solutions for single delaminations may be unconservative

**Assumptions:**

- System of two cracks in a layered plate
- Frictionless contact
- Homogeneous, isotropic and perfectly brittle material

# INTERACTION EFFECTS – QUASI STATIC LOADING MACROSTRUCTURAL RESPONSE

(Andrews and Massabò, Comp. A, 2008)



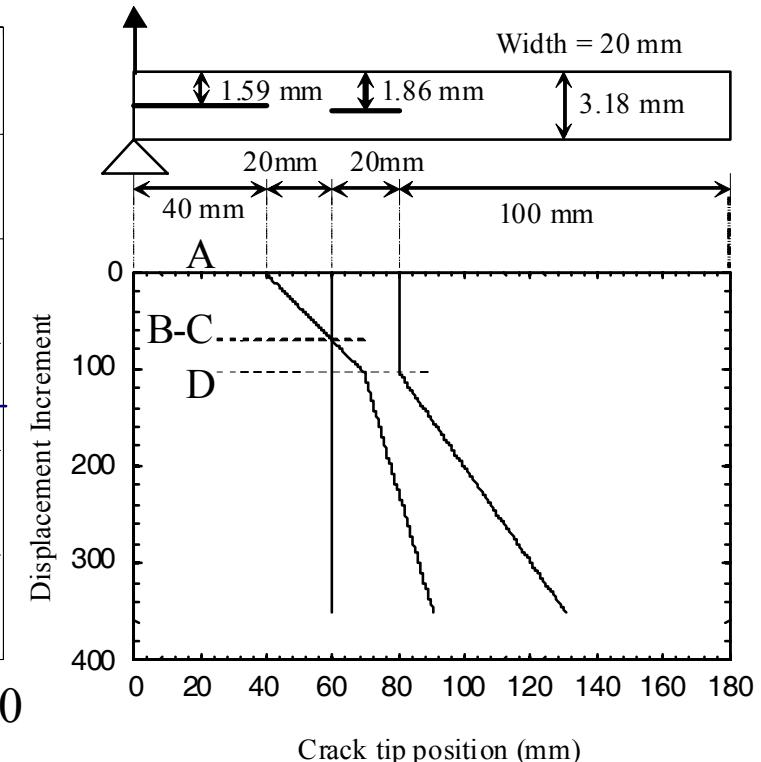
unstable growth of left crack

amplification

shielding

Assumptions:

- Frictionless contact
- Homogeneous, orthotropic, perfectly brittle material



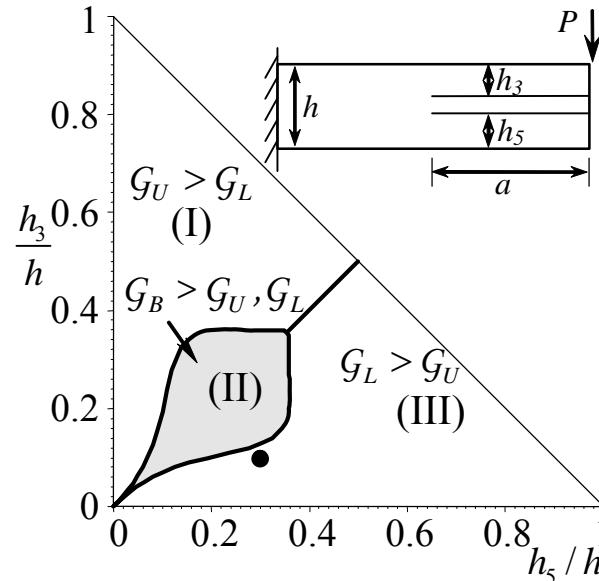
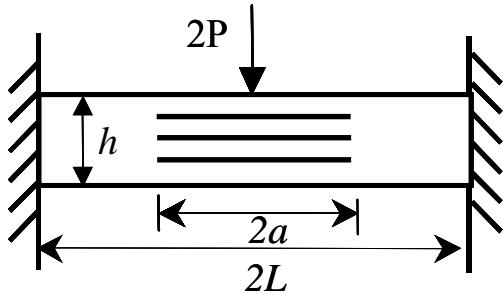
(c)

unstable growth of both cracks

Snap-back and snap-through instabilities due to local amplification and shielding of the crack tip stress fields

# INFLUENCE OF CRACK SPACING ON MULTIPLE DELAMINATION

(Andrews, Massabò & Cox, IJSS, 2006)



Behavioral maps, depending on crack spacing, define regions characterized by equal length growth

- In grey region equality of length is maintained and is stable with respect to length perturbations

→ Is controlled delamination fracture a feasible tool to improve mechanical performance (e.g. energy absorption)?

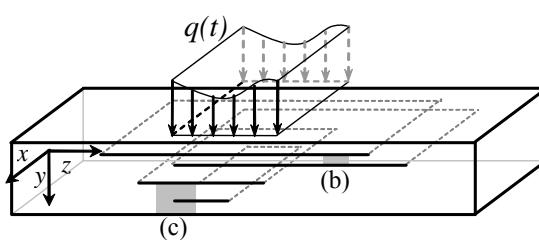
Assumptions:

- frictionless contact
- perfectly brittle material

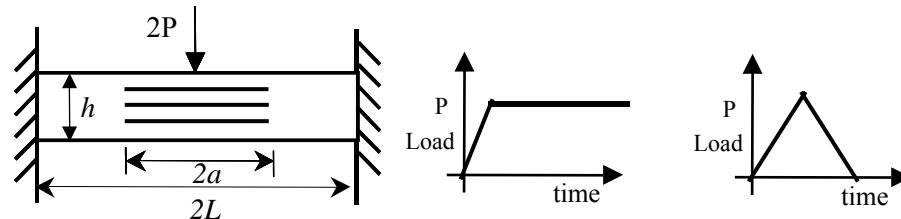
# DYNAMIC INTERACTION EFFECTS IN HOMOGENEOUS SYSTEMS

## STATIONARY DELAMINATIONS

(Andrews, Massabò, Cavicchi & Cox, IJSS, 2009)

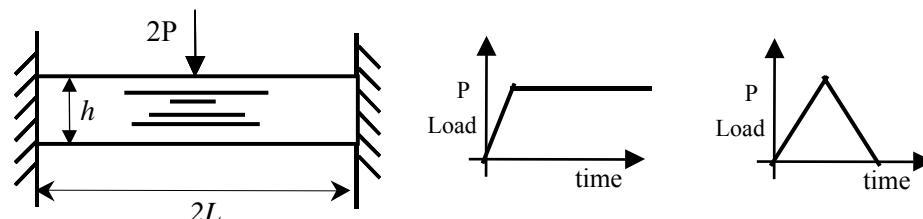


### Equally spaced, equal length delaminations



In phase vibrations of delaminated beams  
Dynamic and interaction effects uncoupled if  
loading pulse is long enough

### Unequally spaced or unequal length delaminations:

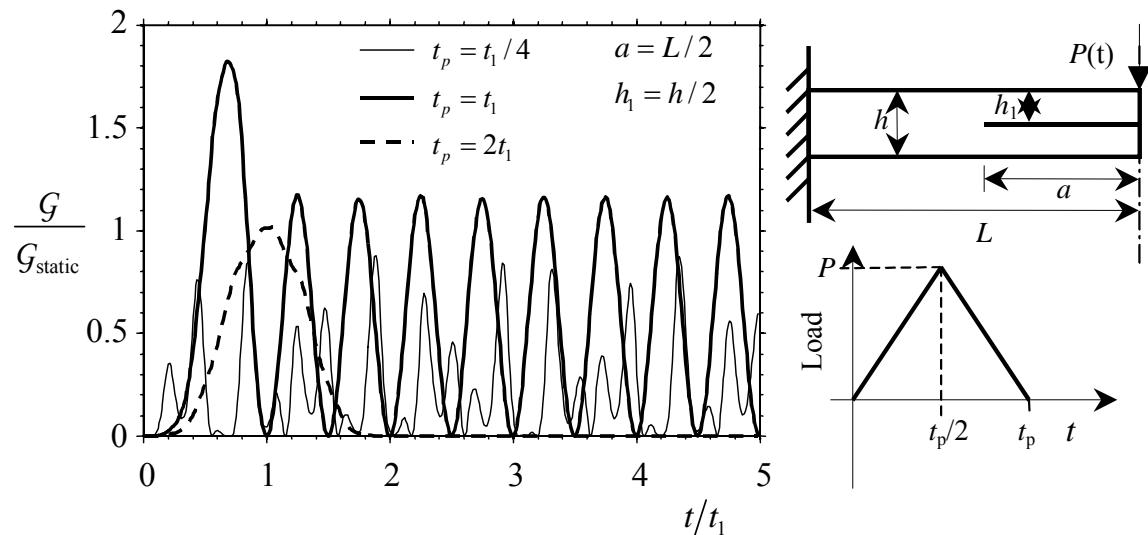


Out of phase vibrations and hammering of  
delaminated beams after load removal

# DYNAMIC INTERACTION EFFECTS - STATIONARY DELAMINATIONS

*equally spaced, equal length delaminations, short pulses*

Time history of normalized energy release rate in systems with one delamination



Assumptions:

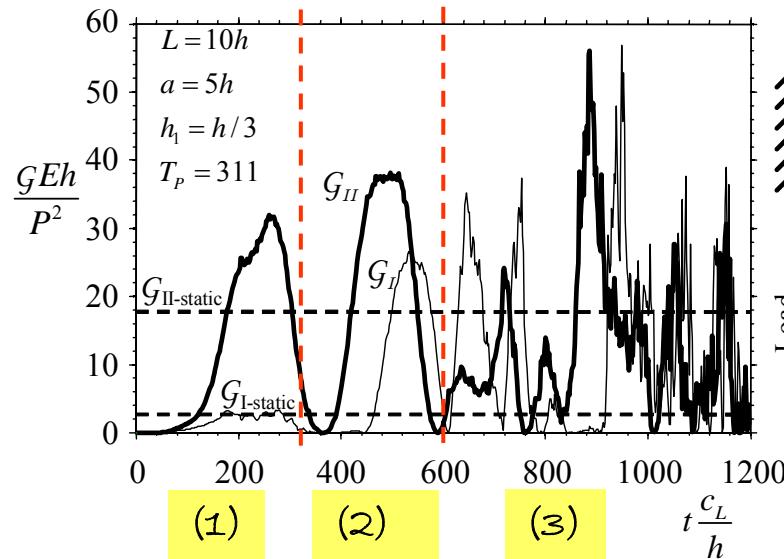
- One crack in clamped-clamped beam
- Frictionless contact
- Homogeneous, isotropic, perfectly brittle

In phase vibrations of delaminated beams  
 Dynamic and interaction effects uncoupled if  
 pulse is long enough

# DYNAMIC INTERACTION EFFECTS - STATIONARY DELAMINATIONS

*unequally spaced, unequal length delaminations, short pulses*

Time history of energy release rate components



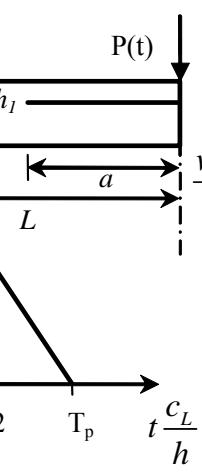
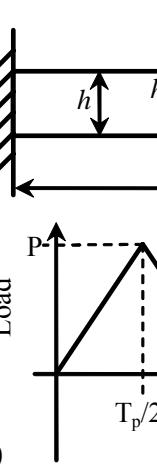
(1)

(2)

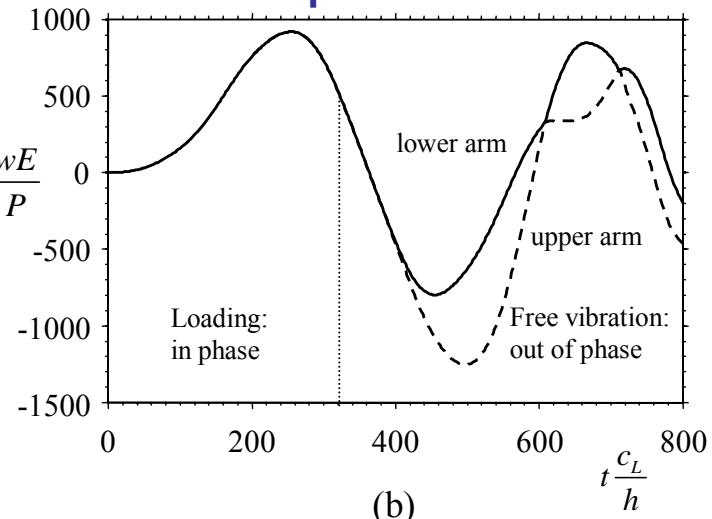
(3)

Forced vibrations

free vibrations



Time history of load point displacements



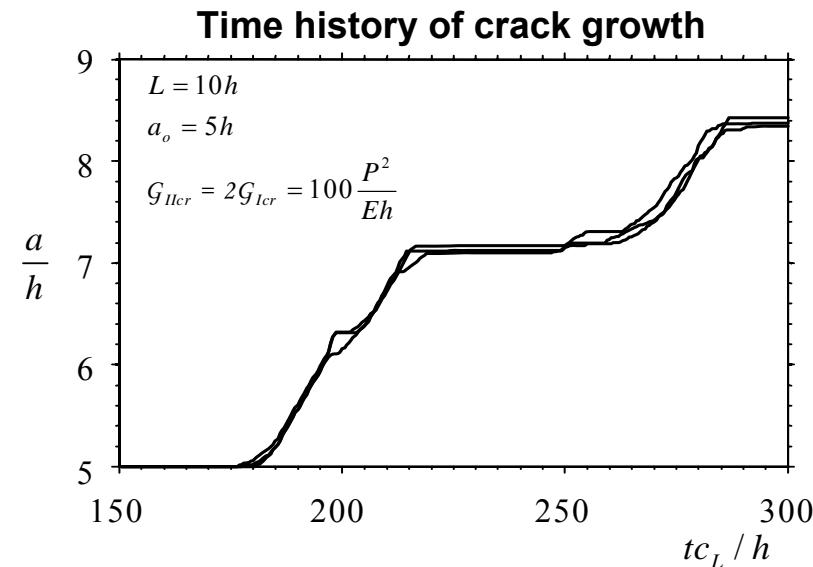
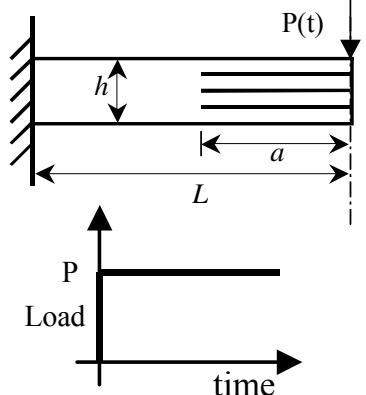
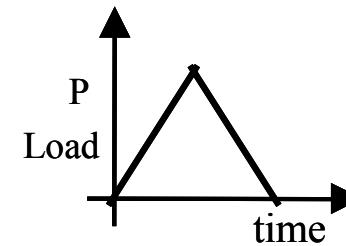
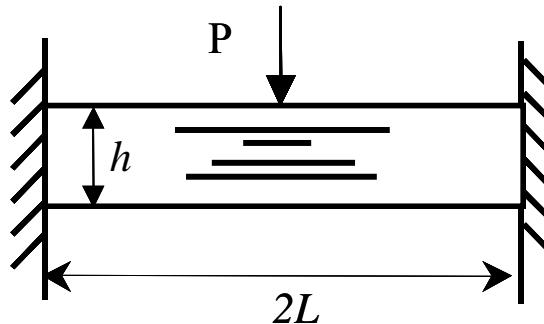
(b)

Assumptions:

- One crack in clamped-clamped beam
- Frictionless contact
- Homogeneous, isotropic, perfectly brittle

Out of phase vibrations, amplifications and hammering of delaminated beams after load removal

## MULTIPLE DYNAMIC DELAMINATION FRACTURE



In homogeneous systems under arbitrary dynamic loading conditions:

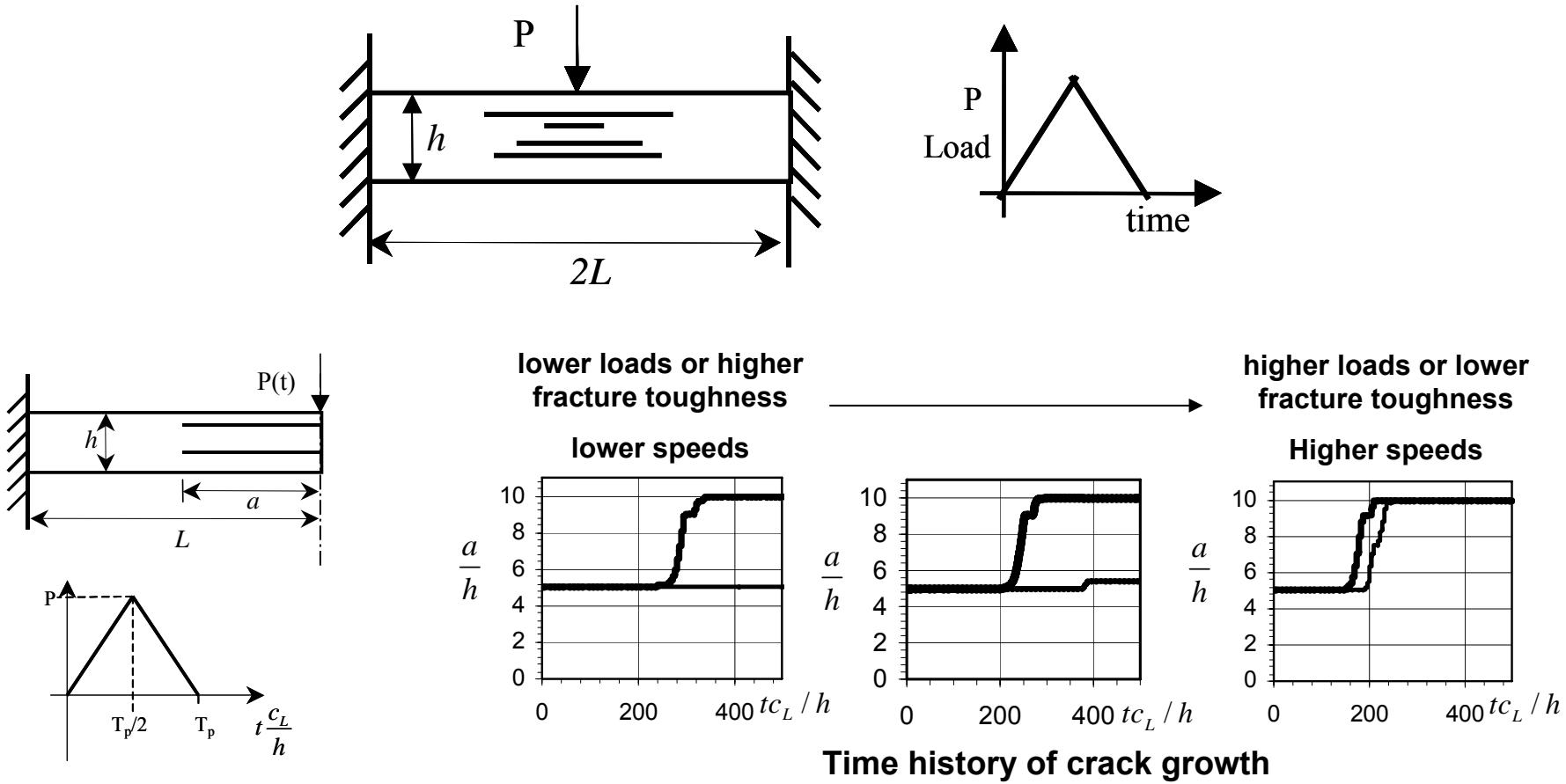
Equally spaced delaminations propagate to equal length configuration and equality of length is then maintained and stable with respect to length perturbations

Assumptions:

- frictionless contact
- perfectly brittle material

(Andrews, Massabò, Cavicchi & Cox, 2008)

# MULTIPLE DYNAMIC DELAMINATION FRACTURE



In homogeneous systems under arbitrary dynamic loading conditions:

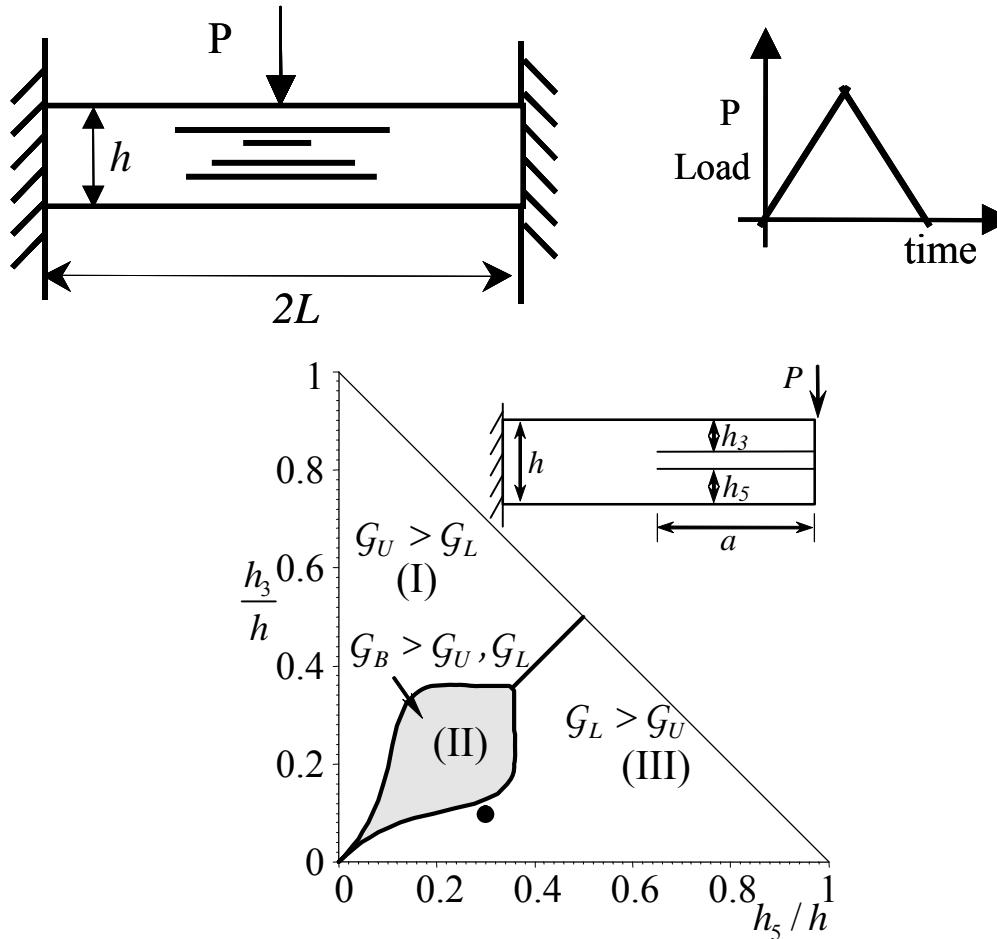
Unequally spaced delaminations: propagation at higher speed of a single dominant crack can be observed; response controlled by crack spacing

Assumptions:

- frictionless contact
- perfectly brittle material

(Andrews, Massabò, Cavicchi & Cox, 2008)

## MULTIPLE DYNAMIC DELAMINATION FRACTURE



Behavioral maps, depending on crack spacing, define regions characterized by equal length growth

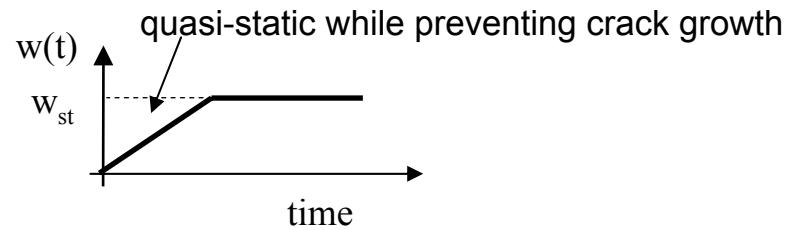
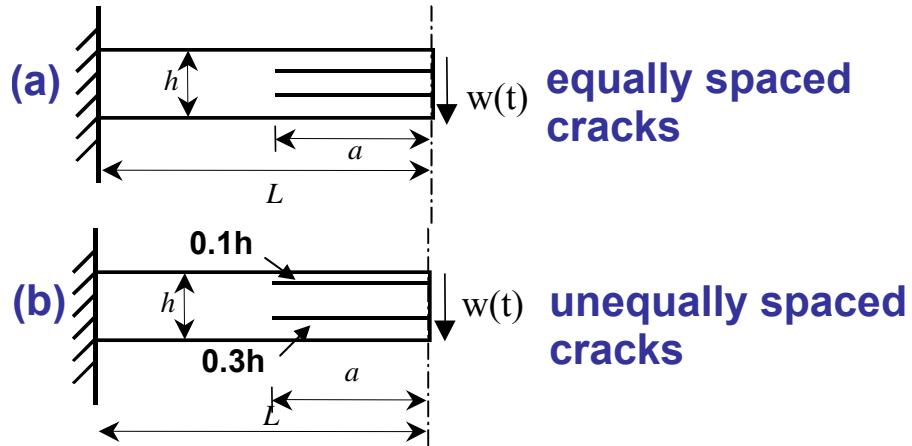
Assumptions:

- frictionless contact
- perfectly brittle material

(Andrews, Massabò & Cox, 2006)

# CONTROLLED DELAMINATION FRACTURE TO IMPROVE MECHANICAL PERFORMANCE AGAINST DYNAMIC LOADINGS

Displacement controlled dynamic growth  
with fixed initial strain energy  $\mathcal{L}$

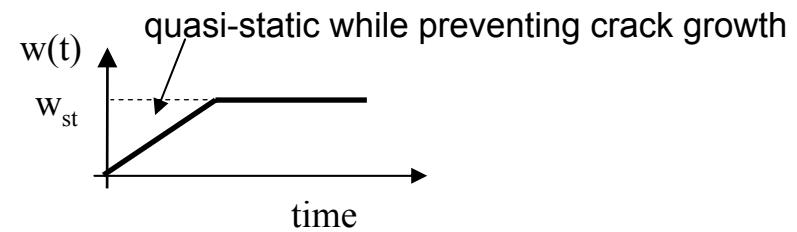
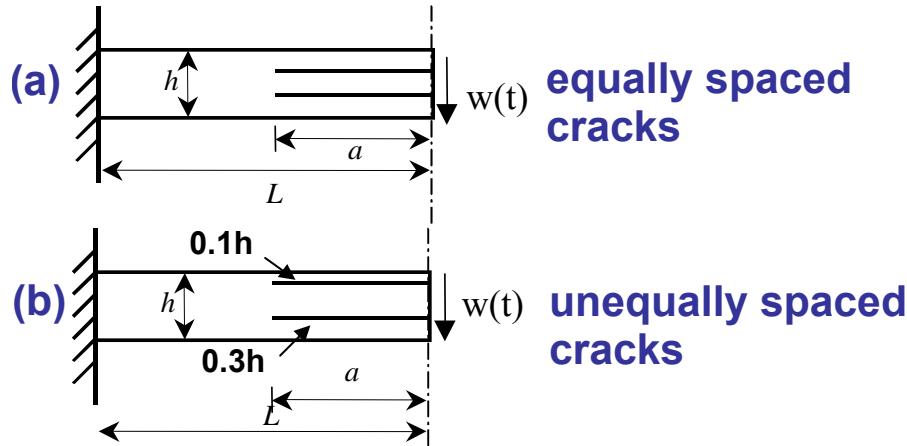


## Assumptions:

- $G_{cr}h / \mathcal{L}$  leading to small / moderate crack speeds
- Frictionless contact
- Homogeneous, isotropic, perfectly brittle material

# CONTROLLED DELAMINATION FRACTURE TO IMPROVE MECHANICAL PERFORMANCE AGAINST DYNAMIC LOADINGS

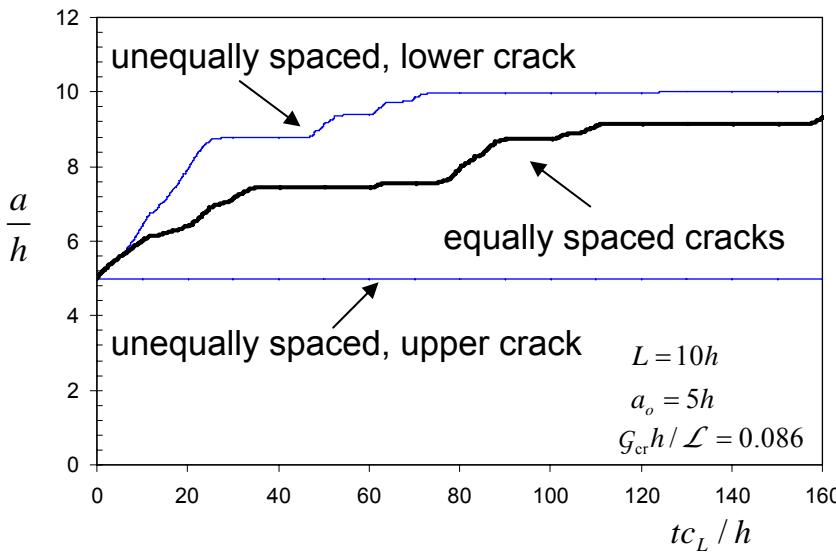
Displacement controlled dynamic growth with fixed initial strain energy  $\mathcal{L}$



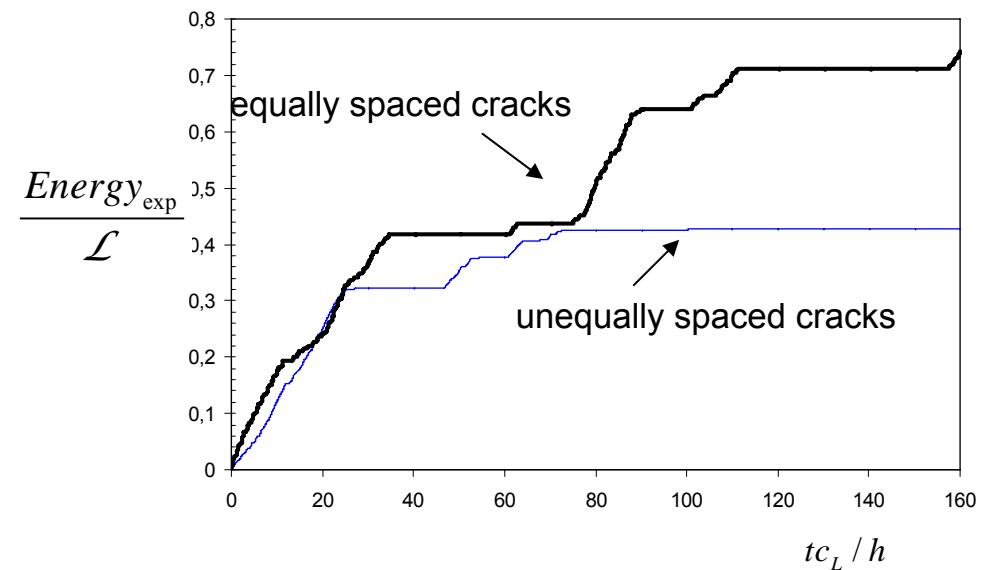
## Assumptions:

- $G_{cr}h / \mathcal{L}$  leading to small / moderate crack speeds
- Frictionless contact
- Homogeneous, isotropic, perfectly brittle material

## Time history of crack growth

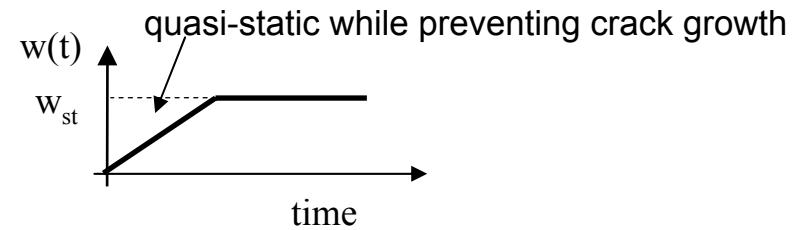
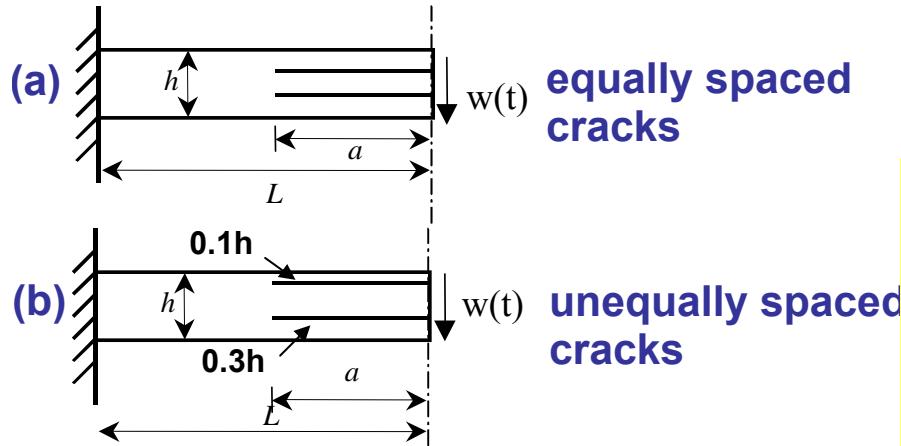


## Time history of expended energy



# CONTROLLED DELAMINATION FRACTURE TO IMPROVE MECHANICAL PERFORMANCE AGAINST DYNAMIC LOADINGS

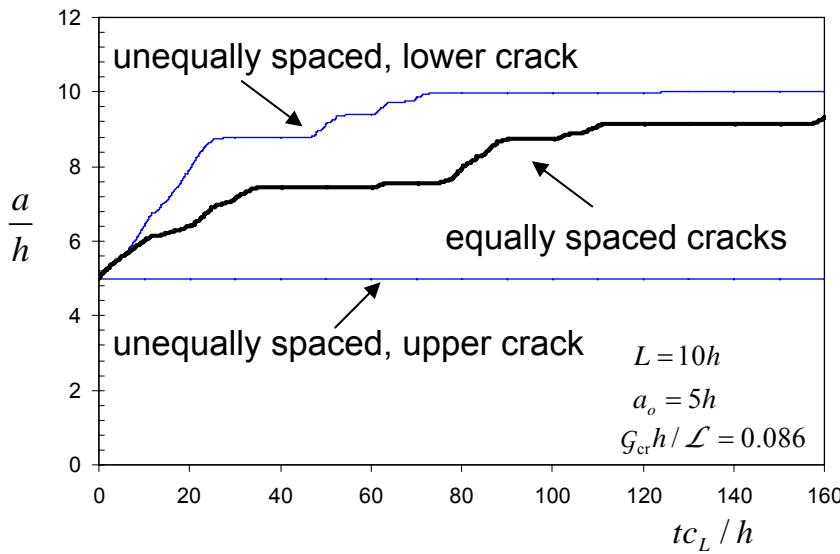
Displacement controlled dynamic growth with fixed initial strain energy  $\mathcal{L}$



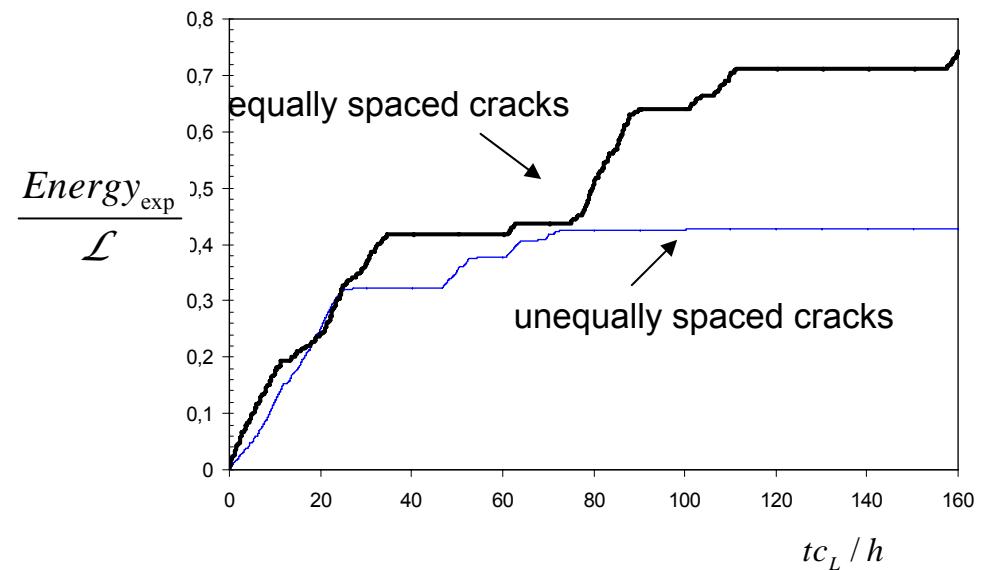
Energy absorption through multiple delamination fracture in laminated plates can be optimized via a material design that favour crack formation along predefined planes

eds

Time history of crack growth



Time history of expended energy

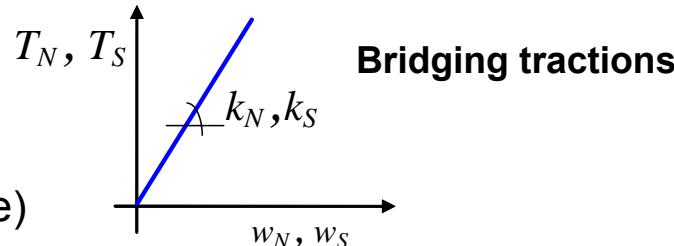


# INFLUENCE OF CRACK BRIDGING MECHANISMS STATIONARY DELAMINATIONS

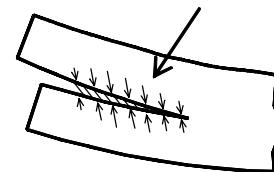
**Linear proportional bridging**

$$k^N, k^S = 0.01E/h$$

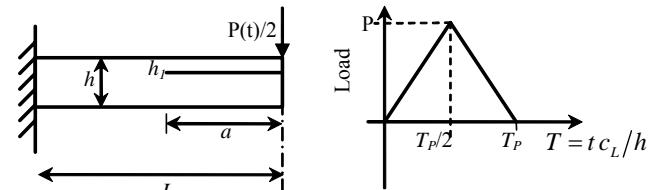
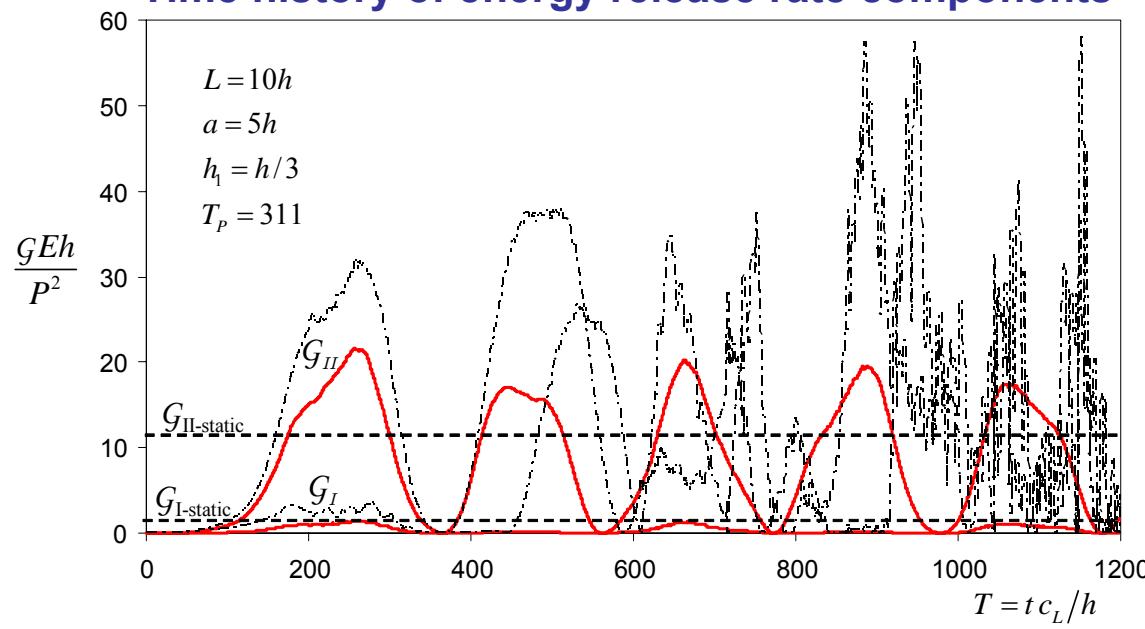
(values for a typical stitched laminate)



continuous bridging tractions



**Time history of energy release rate components**

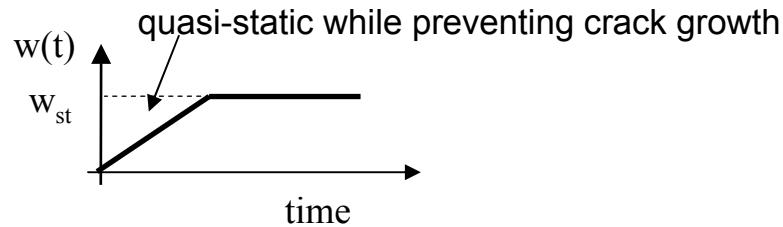


Bridging mechanisms:

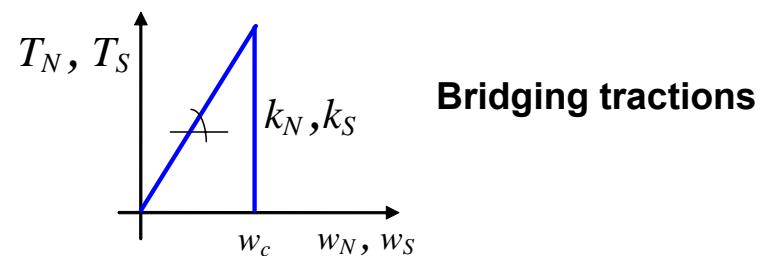
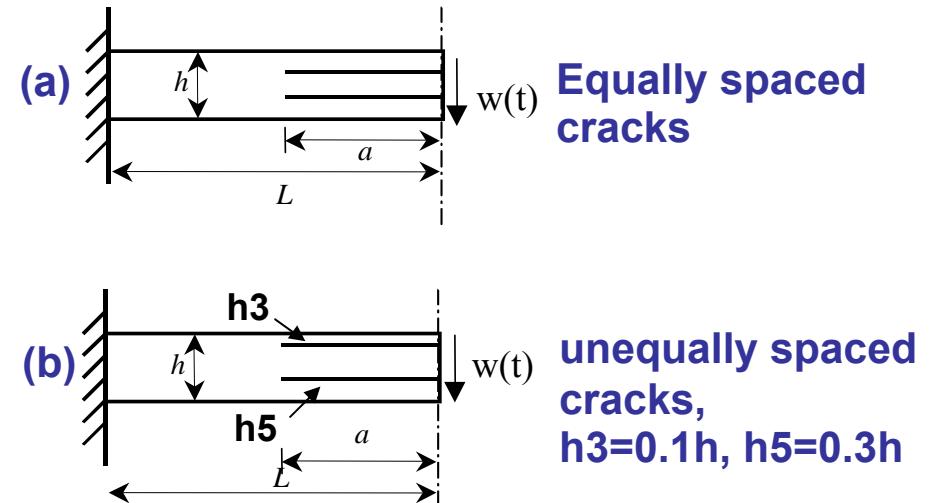
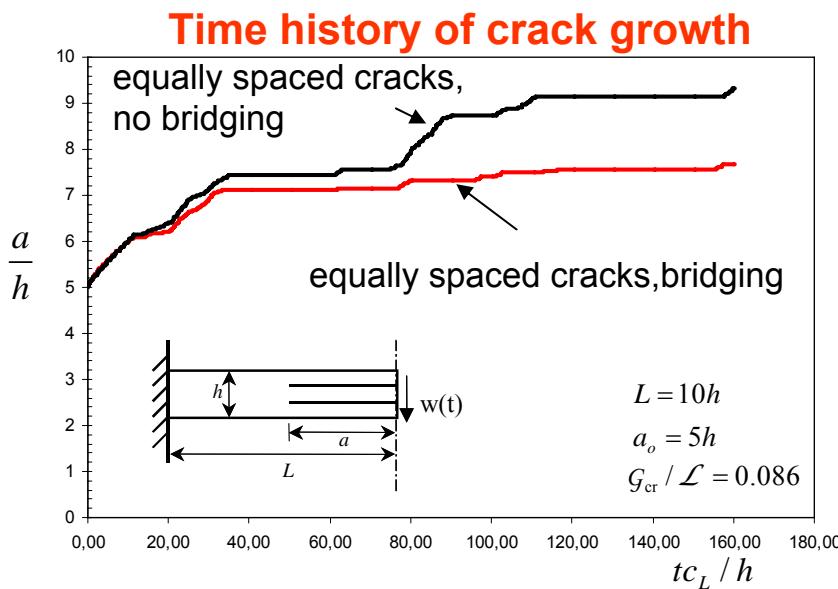
- shield crack tip field from applied loading
- Problem becomes mode II
- stabilize free vibration phase

# ENERGY ABSORPTION THROUGH MULTIPLE DELAMINATION INFLUENCE OF CRACK BRIDGING MECHANISMS

Displacement controlled dynamic growth  
with fixed initial strain energy  $\mathcal{L}$



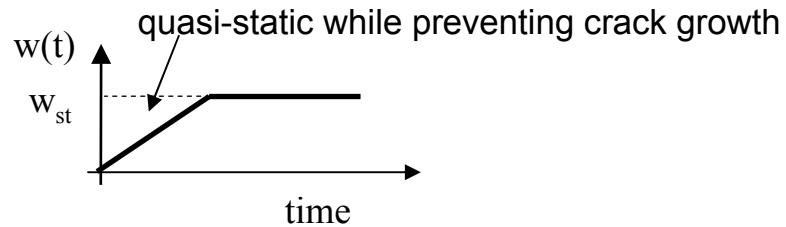
Results for:  $G_{cr}h / \mathcal{L} = 0.086$  leading to  
small / moderate crack speeds



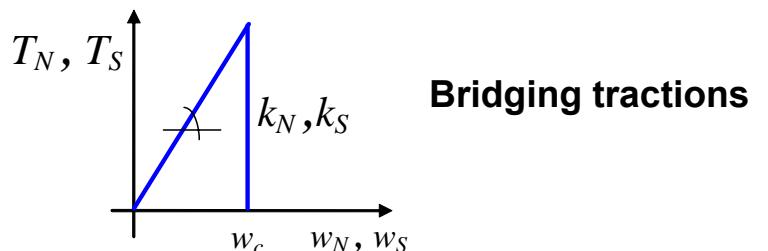
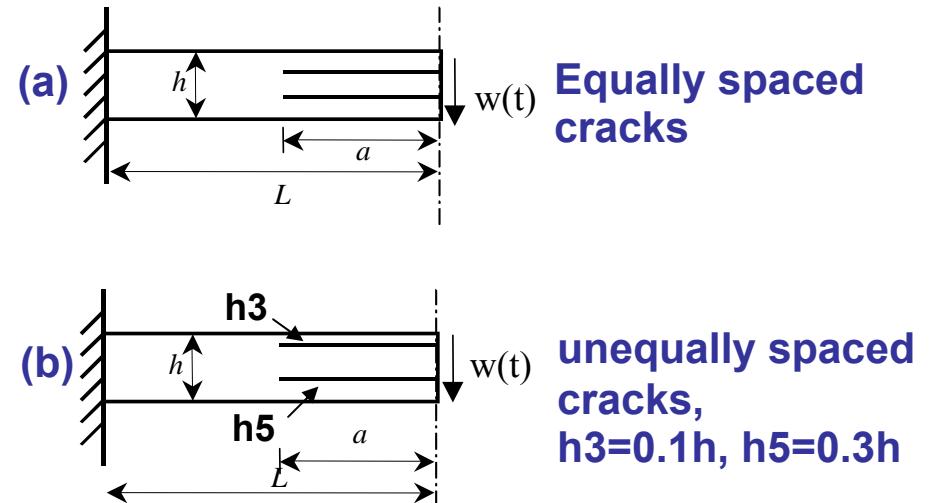
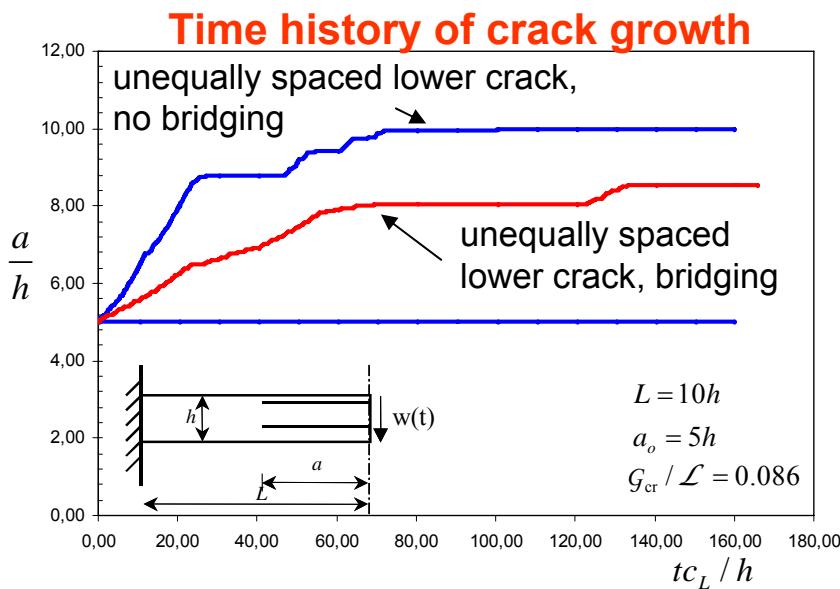
$k^N, k^S = 0.01E/h$   
 $w_c = 0.1h$   
(values for a typical stitched laminate)

# ENERGY ABSORPTION THROUGH MULTIPLE DELAMINATION INFLUENCE OF CRACK BRIDGING MECHANISMS

Displacement controlled dynamic growth  
with fixed initial strain energy  $\mathcal{L}$



Results for:  $G_{cr}h / \mathcal{L} = 0.086$  leading to  
small / moderate crack speeds



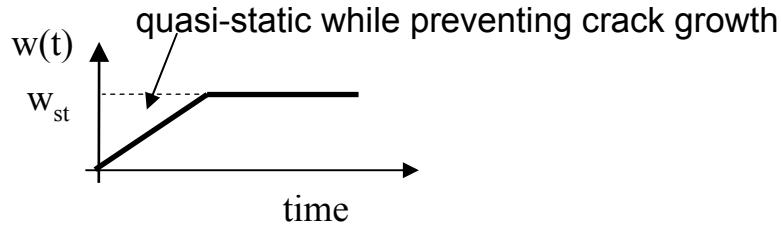
$$k^N, k^S = 0.01E/h$$

$$w_c = 0.1h$$

(values for a typical stitched laminate)

# ENERGY ABSORPTION THROUGH MULTIPLE DELAMINATION INFLUENCE OF CRACK BRIDGING MECHANISMS

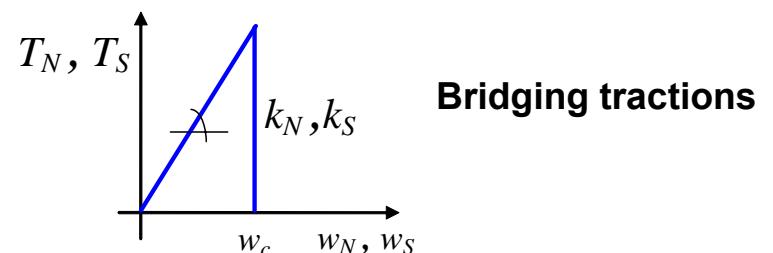
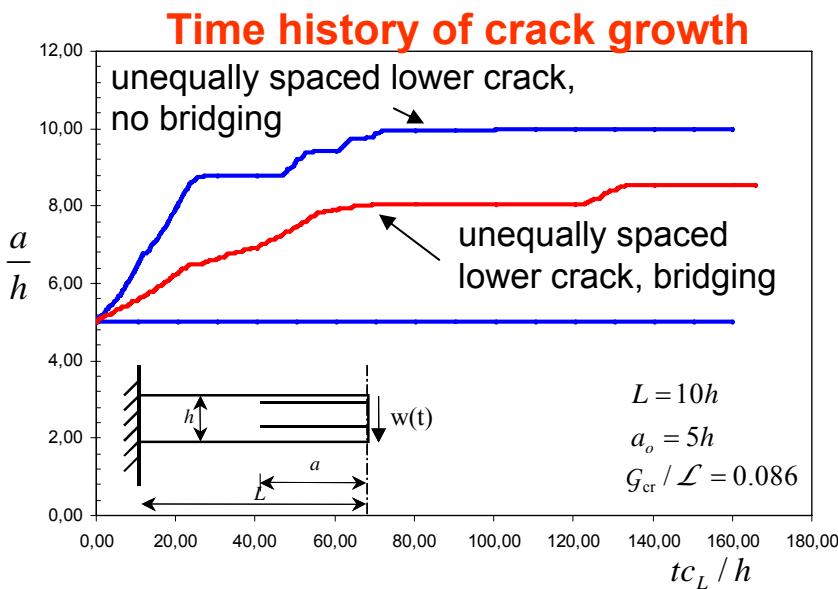
Displacement controlled dynamic growth  
with fixed initial strain energy  $\mathcal{L}$



Results for:  $G_{cr}h / \mathcal{L} = 0.086$  leading to  
small / moderate crack speeds

Bridging mechanisms in the regime of low to moderate crack speeds:

- reduce crack speed
- may lead to crack arrest
- minimize differences in the response of systems with equally and unequally spaced cracks



$$k^N, k^S = 0.01E/h$$

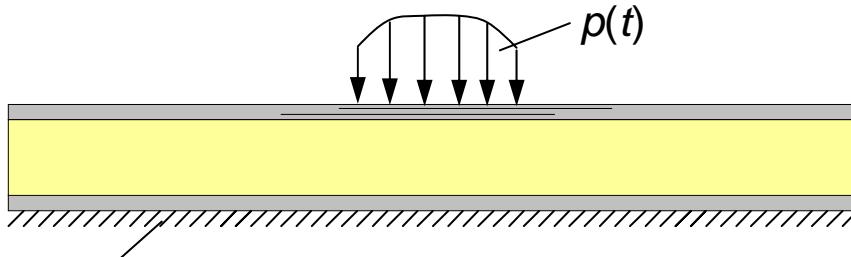
$$w_c = 0.1h$$

(values for a typical stitched laminate)

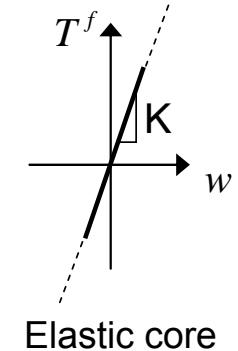
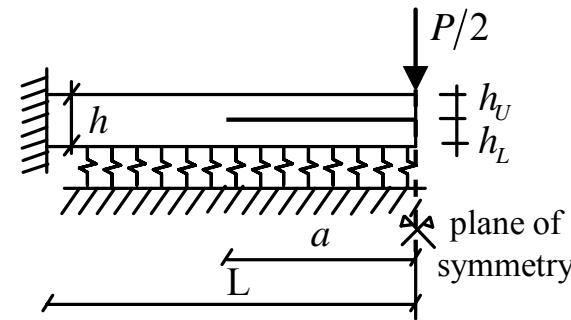
# INTERACTION EFFECTS IN SANDWICH BEAMS

## SHIELDING OF FRACTURE PARAMETERS AND ENERGY BARRIERS

(quasi static loading, elastic foundation)



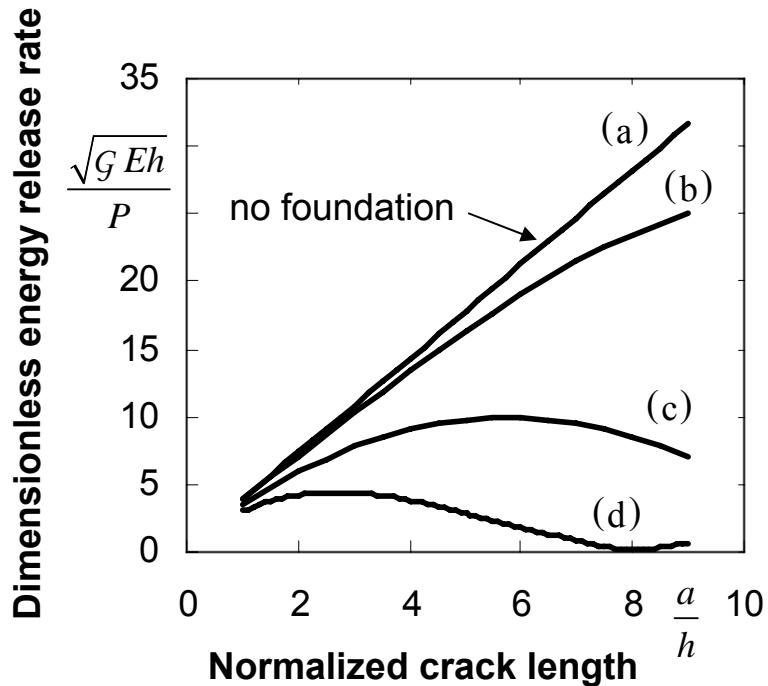
Rigid Support



$$L/h = 10$$

$$h_U/h = 0.5$$

Perfect core/skin interface



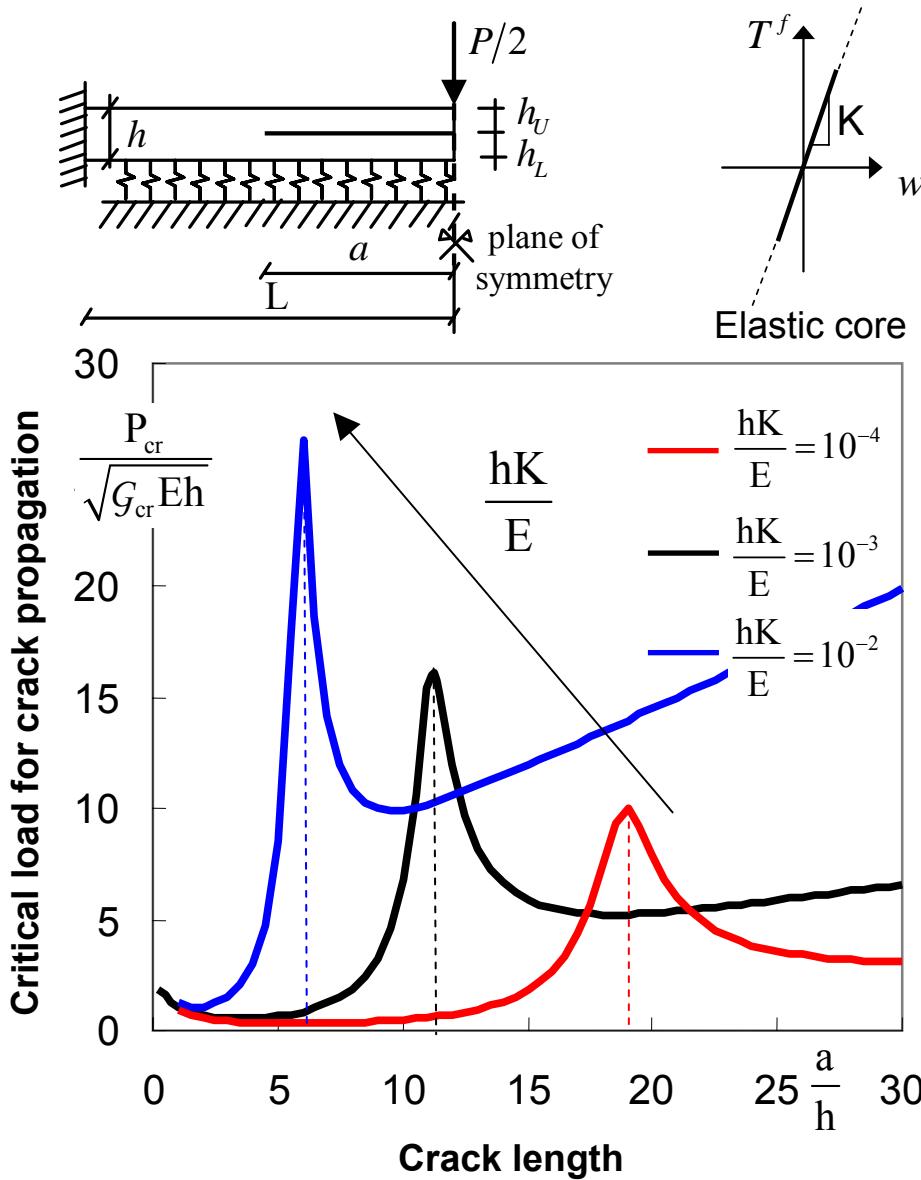
- (a)  $\frac{hK}{E} = 0$
- (b)  $\frac{hK}{E} = 10^{-4}$
- (c)  $\frac{hK}{E} = 10^{-3}$
- (d)  $\frac{hK}{E} = 10^{-2}$

- Face-core interactions shield crack tips from applied loads
- Transition in energy release rate on increasing the core-skin stiffness ratio
- Minimum in  $G$  defines energy barrier to crack propagation

# INTERACTION EFFECTS IN SANDWICH BEAMS

## SHIELDING OF FRACTURE PARAMETERS AND ENERGY BARRIERS

(quasi static loading, elastic foundation)



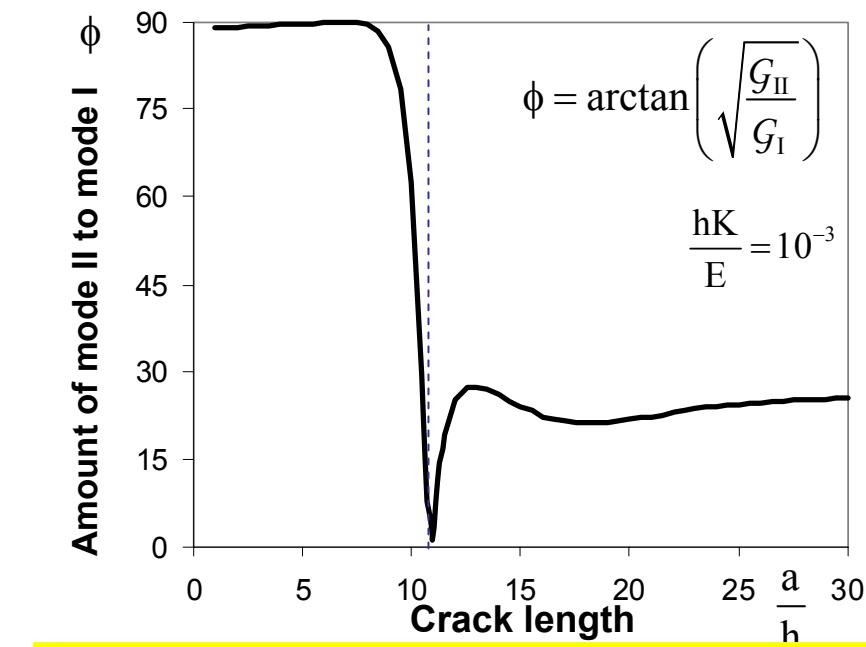
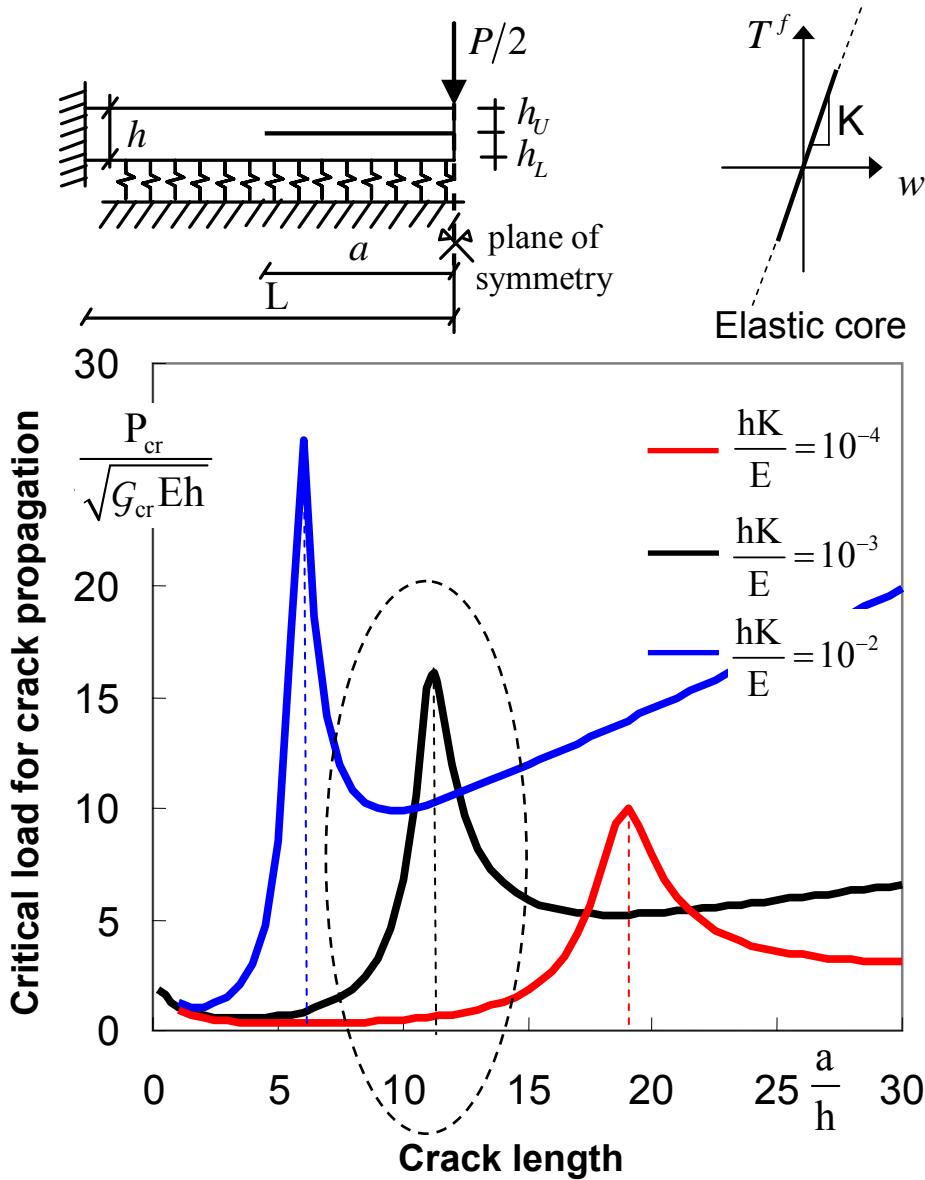
$L/h = 100$       Brittle skin,  $G_{cr}$   
 $h_U/h = 0.5$       Perfect core/skin interface  
 $G = G_{cr}$  (crack growth criterion)

-Energy barrier for a characteristic length of the crack depending on the core-skin stiffness ratio

# INTERACTION EFFECTS IN SANDWICH BEAMS

## SHIELDING OF FRACTURE PARAMETERS AND ENERGY BARRIERS

(quasi static loading, elastic foundation)

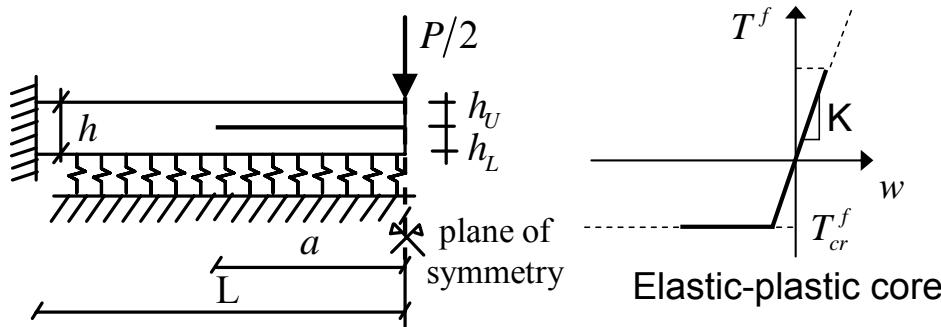


-Energy barrier for a characteristic length of the crack depending on the core-skin stiffness ratio

- Sudden transition from mode II to mixed mode fracture at the barrier

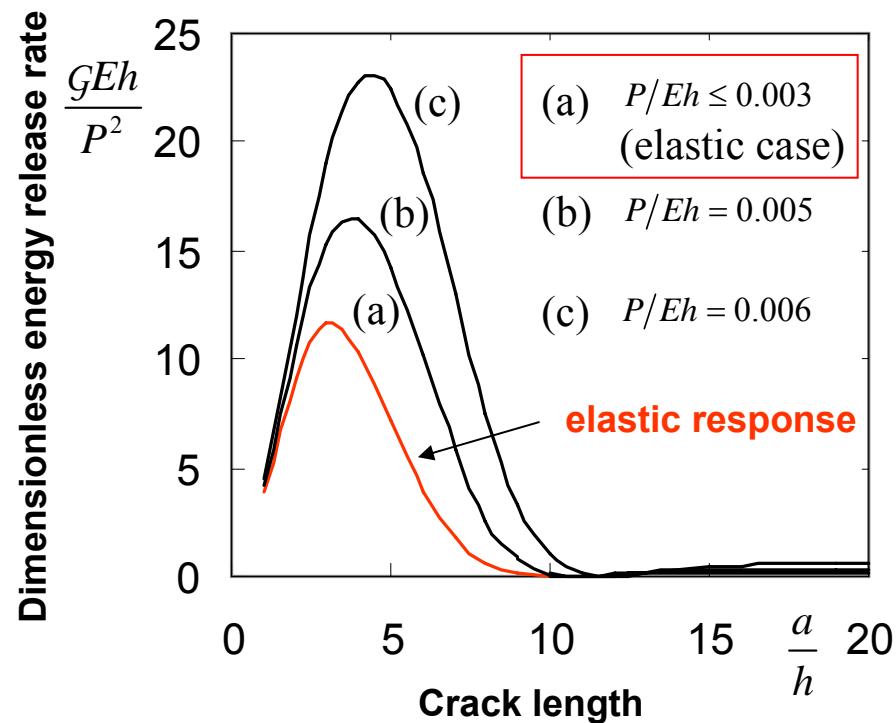
# INFLUENCE OF CORE PLASTICITY ON FRACTURE PARAMETERS AND MECHANICAL RESPONSE

(static loading, elastic-plastic foundation)



$$L/h = 100 \quad \frac{T_{cr}^f}{E} = 10^{-3} \quad \frac{hK}{E} = 10^{-3}$$

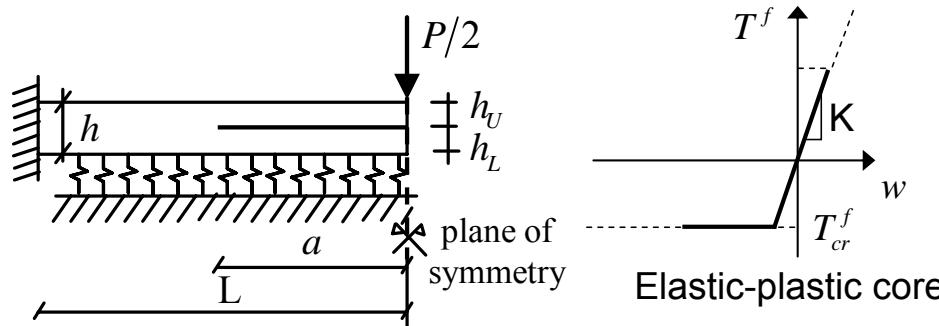
Perfect core/skin interface



- Plasticity of the core reduces elastic shielding and modifies dependence of energy release rate on applied load

# INFLUENCE OF CORE PLASTICITY ON FRACTURE PARAMETERS AND MECHANICAL RESPONSE

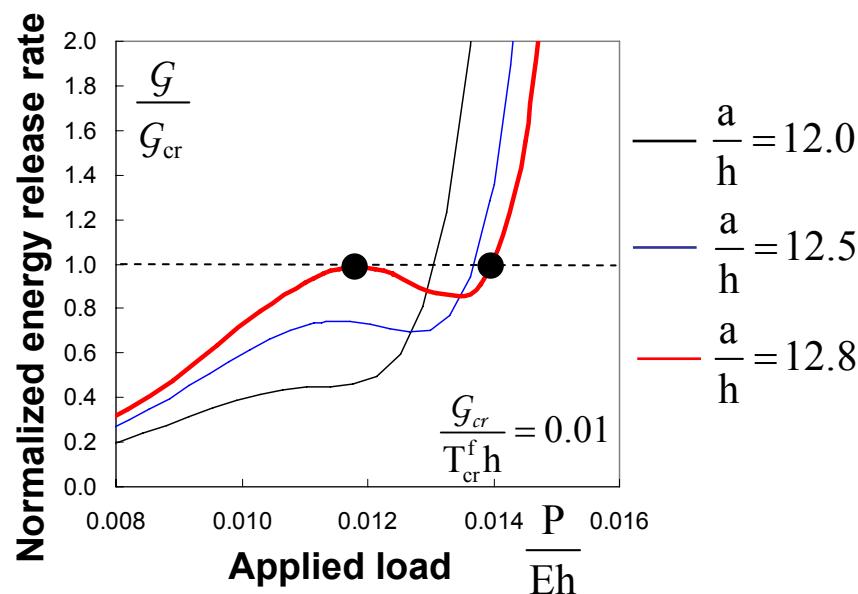
(static loading, elastic-plastic foundation)



$$L/h = 100 \quad G = G_{cr} \text{ (crack growth criterion)}$$

$$h_U/h = 0.5 \quad \frac{G_{cr}}{Eh} = 10^{-5} \quad \frac{hK}{E} = 10^{-3}$$

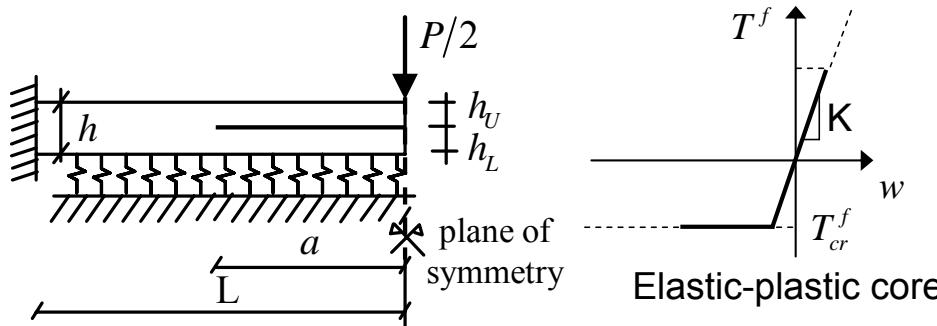
Perfect core/skin interface



- The problem solution is not unique
- The response depends on the loading and propagation histories

# INFLUENCE OF CORE PLASTICITY ON FRACTURE PARAMETERS AND MECHANICAL RESPONSE

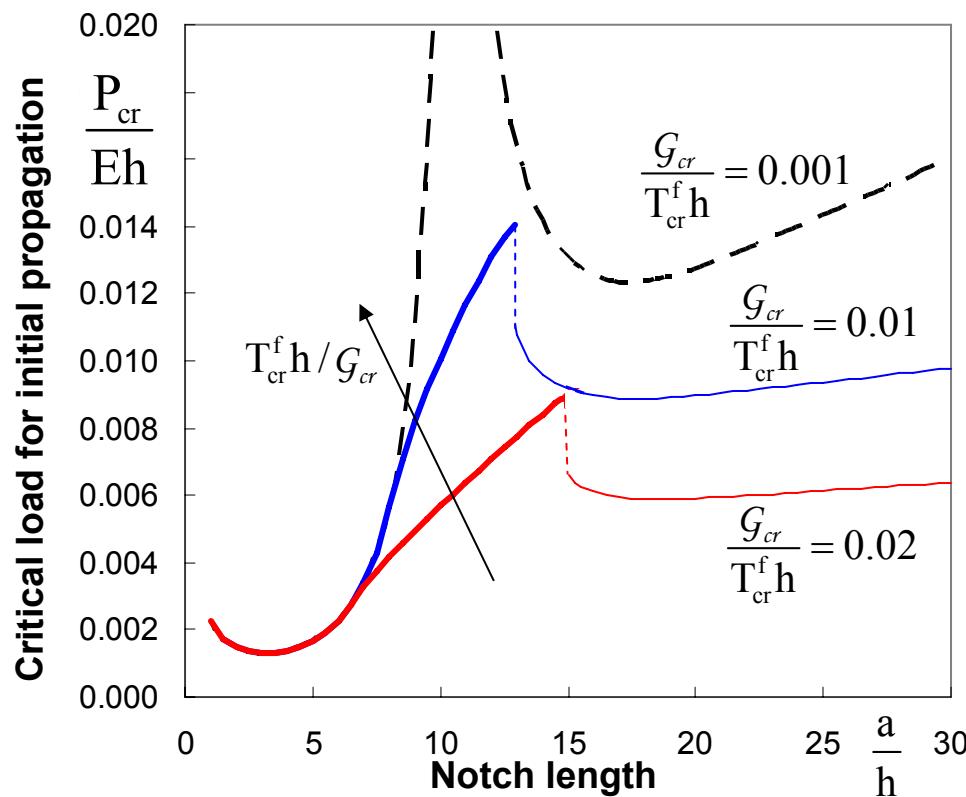
(static loading, elastic-plastic foundation)



$$L/h = 100 \quad G = G_{cr} \text{ (crack growth criterion)}$$

$$h_U/h = 0.5 \quad \frac{G_{cr}}{Eh} = 10^{-5} \quad \frac{hK}{E} = 10^{-3}$$

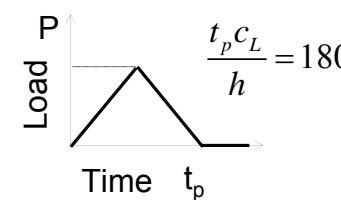
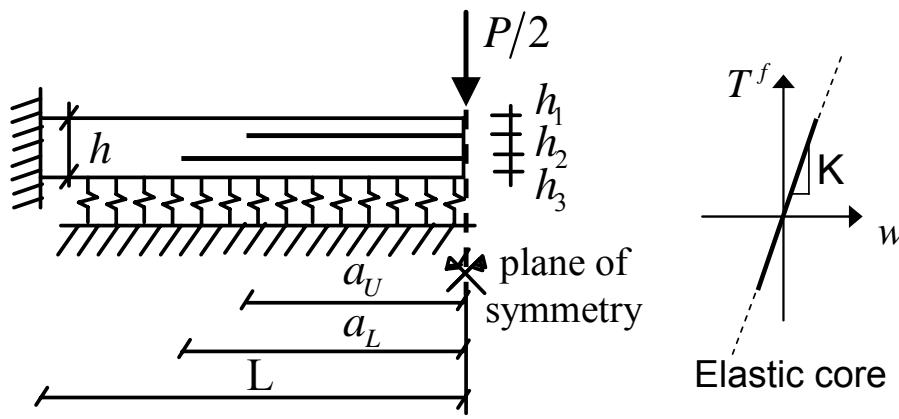
Perfect core/skin interface



- The problem solution is not unique
- The response depends on the loading and propagation histories

# MULTIPLE DYNAMIC DELAMINATION FRACTURE OF THE SKIN

(dynamic loading, elastic foundation)



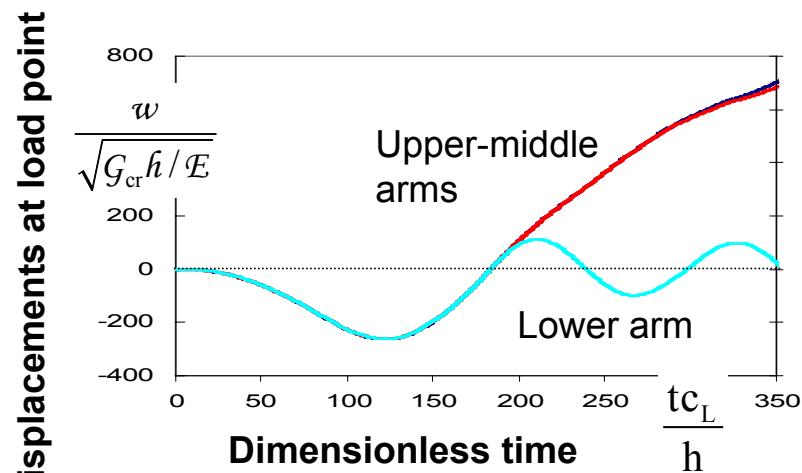
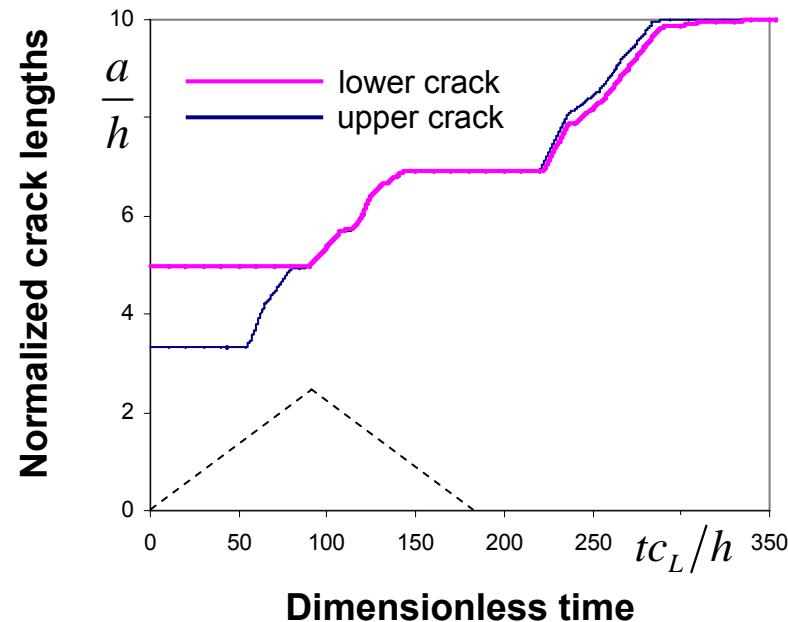
$$\frac{t_p c_L}{h} = 180$$

$$L/h = 10$$

$$h_U/h = 1/3 \quad h_L/h = 1/3$$

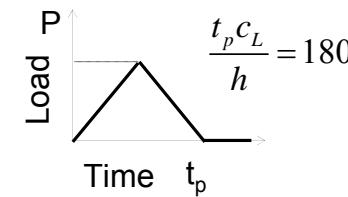
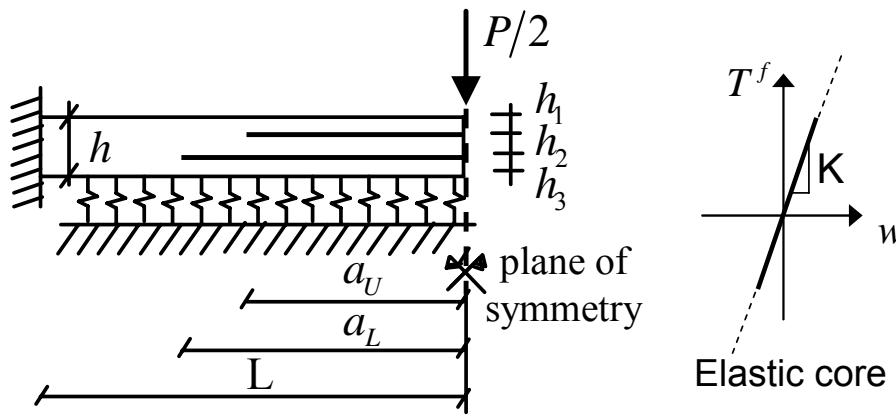
$$\frac{P_{max}}{\sqrt{G_{cr} Eh}} = 1 \quad \frac{hK}{E} = 10^{-3}$$

$G \geq G_{cr}$  (crack growth criterion)



# MULTIPLE DYNAMIC DELAMINATION FRACTURE OF THE SKIN

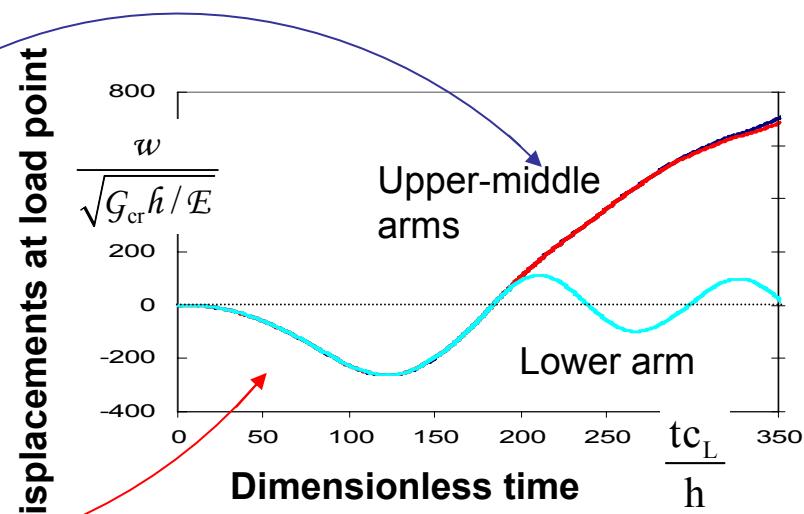
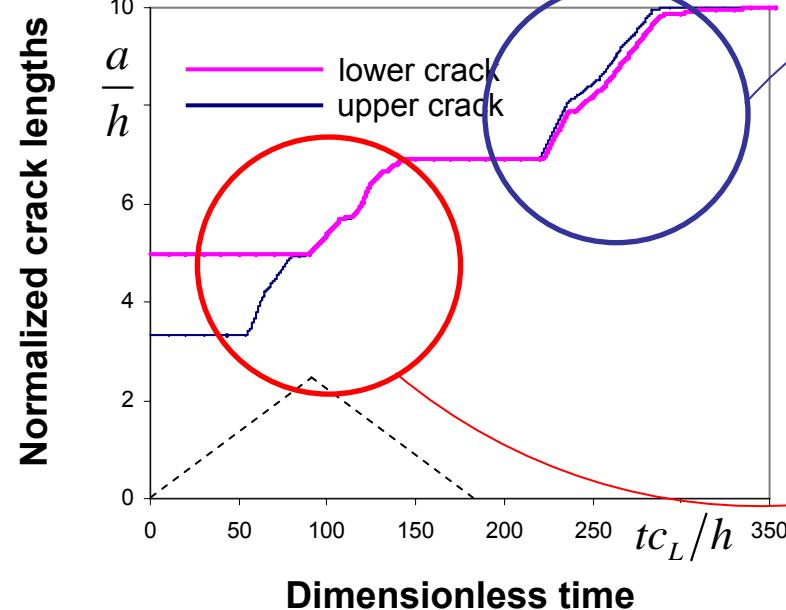
(dynamic loading, elastic foundation)



$$\begin{aligned} L/h &= 10 \\ h_U/h &= 1/3 \quad h_L/h = 1/3 \end{aligned}$$

$$\frac{P_{max}}{\sqrt{G_{cr} Eh}} = 1 \quad \frac{hK}{E} = 10^{-3}$$

$G \geq G_{cr}$  (crack growth criterion)



Dynamic loading and inertial effects trigger mechanisms of multiple crack propagation that are absent in quasi-static cases

## CONCLUSIONS

- Static and dynamic interaction effects of multiple damage mechanisms in composite laminates, multilayered systems and composite sandwiches have been investigated
- Interaction effects on fracture parameters include: amplification and shielding of the energy release rate of one crack due to the presence of other cracks, modification of mode ratios, crack shielding and energy barriers due to the presence of elastic cores
- Interaction effects on macrostructural response include: snap-back and snap-through instabilities, hyper-strength, crack pull along
- Controlled delamination fracture can be a viable tool to improve damage/impact tolerance and energy absorption
- Crack bridging mechanisms stabilize the response of multiply delaminated beams in the free vibration phase after the removal of the load; they reduce crack speed and may lead to crack arrest
- Plasticity of the core reduces shielding of the fracture parameters; the problem solution becomes non unique and the response dependent on loading and propagation history; work is in progress ....